



Address **240 Canterbury Dr**
Austin, TX 78737

Get Google Maps on your phone
Text the word "GMAPS" to 466453

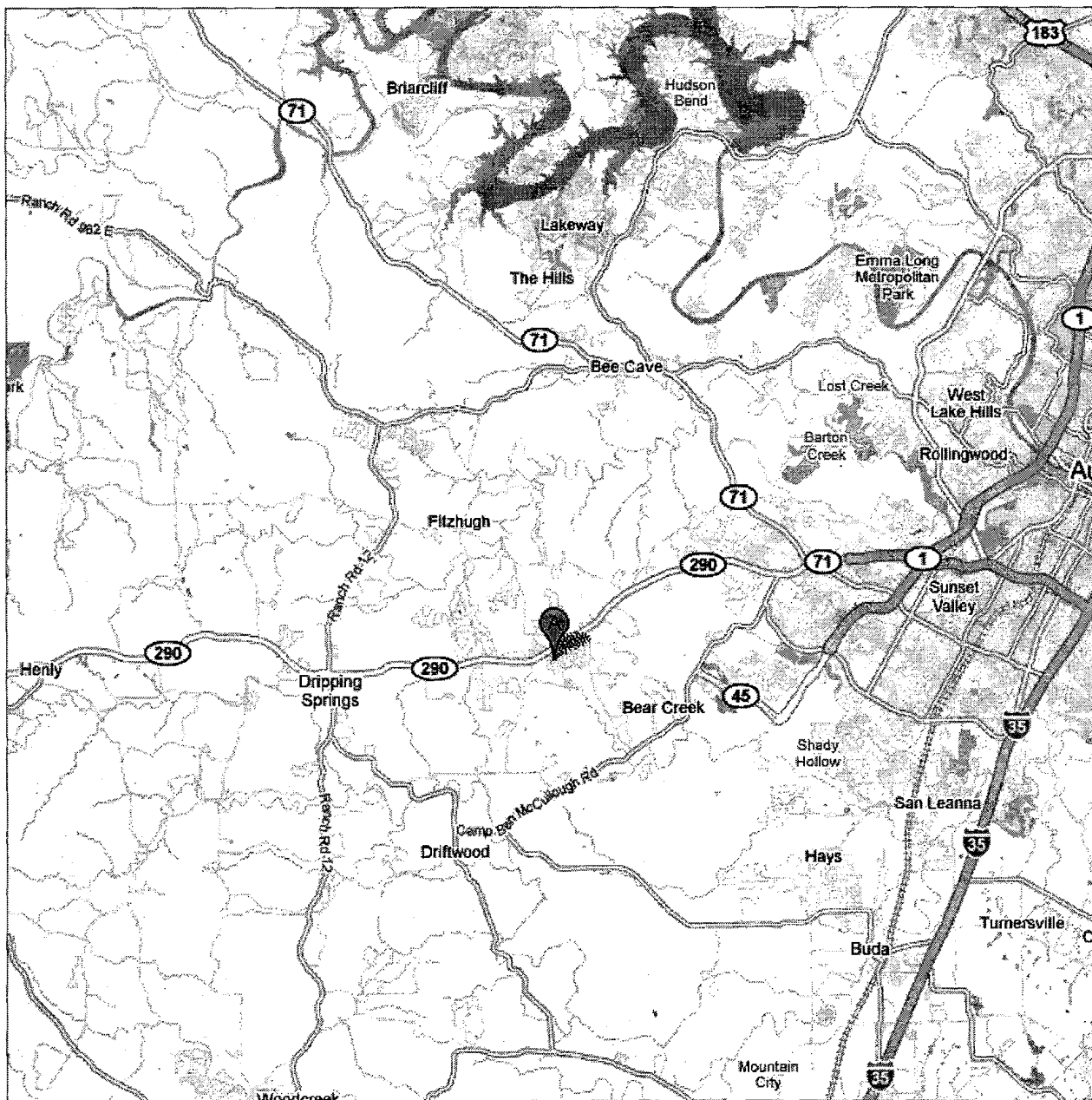


EXHIBIT 1

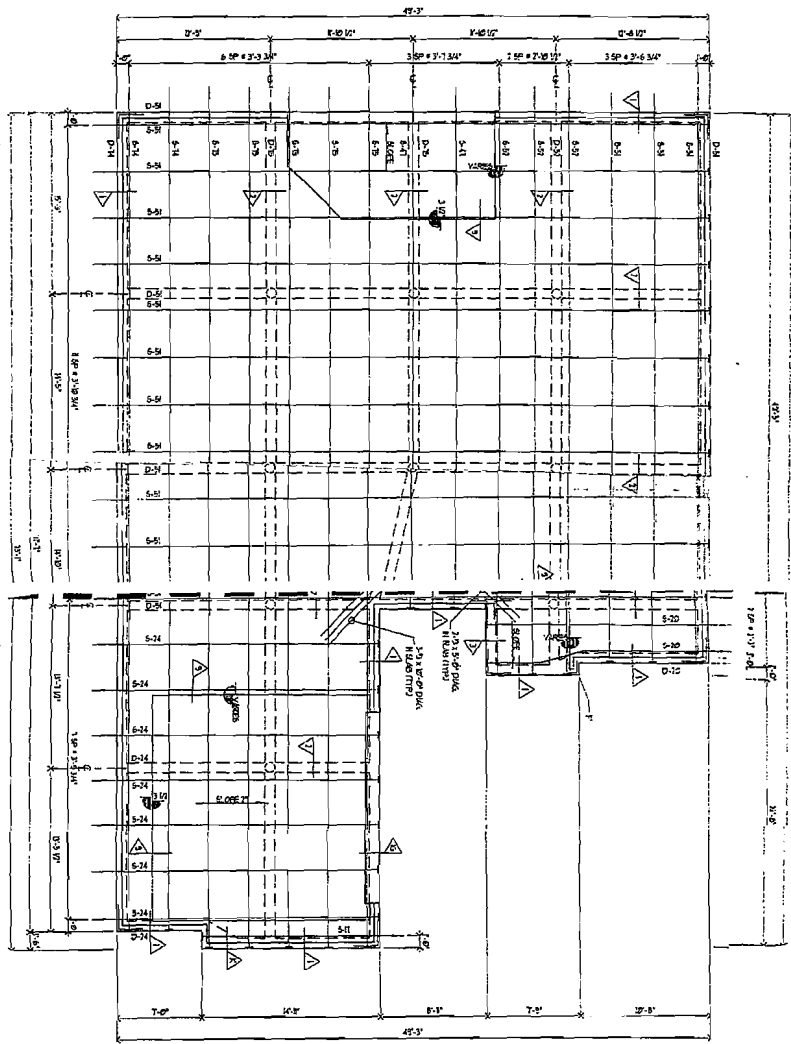
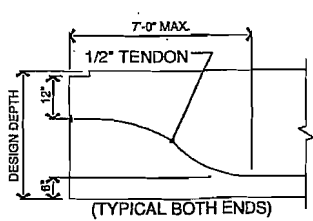


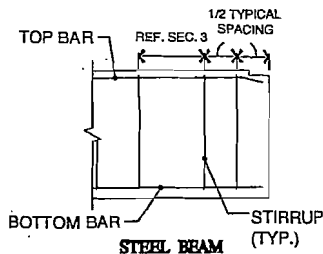
EXHIBIT 2

<input checked="" type="checkbox"/> FILL APPROVED ON THIS SITE		<input type="checkbox"/> FILL UNAPPROVED ON THIS SITE		IF ONE OF THESE BOXES IS CHECKED, SEE NOTES ON SHEET 2 FOR ADDITIONAL FILL INFO.	
INSPECTIONS INSPECTOR: _____ DATE: _____ BY: _____		ESTIMATES 3 Estimate No. _____ Estimate Date: _____ No. of Pages: _____ No. of Sheets: _____		LEGEND [Symbol] APPROVED [Symbol] UNAPPROVED [Symbol] UNKNOWN	
FOUNDATION PLAN SCALE: 1/8" = 1'-0" S1 OF 2		CAPITAL HOMES 240 CENTERBURY DRIVE CANTON, TEXAS 75741 PHONE: 817-481-1000 FAX: 817-481-1005 WWW: WWW.CAPITALHOMES.COM		THE SEAL APPEARING ON THIS DOCUMENT WAS OBTAINED BY THE ENGINEER UNDER THE PROVISIONS OF THE PROFESSIONAL ENGINEERING ACT, CHAPTER 901, TEXAS STATUTES, EFFECTIVE SEPTEMBER 1, 1999. STATE OF TEXAS ENGINEERING BOARD LICENSE NO. 115-00	

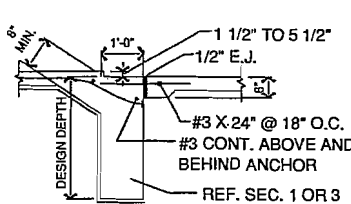


CABLE BEAM

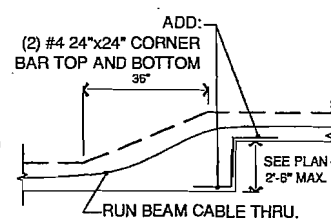
TYPICAL BEAM (SIDE VIEW)



STEEL BEAM



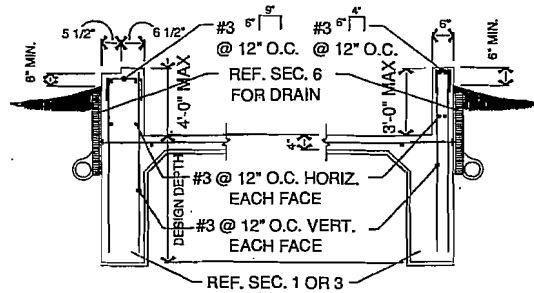
GARAGE APRON - (G)



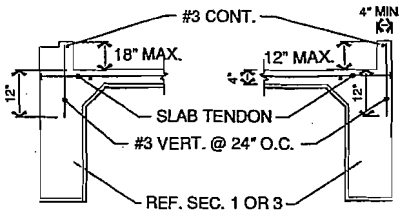
PLAN VIEW

SECTION - (K)

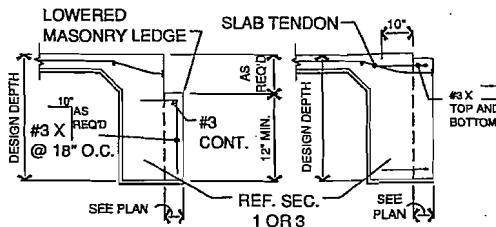
OMIT MASS WHERE NEEDED
6 MIL. POLY (TYP)
1/2" DIA. TENDON (TYP)



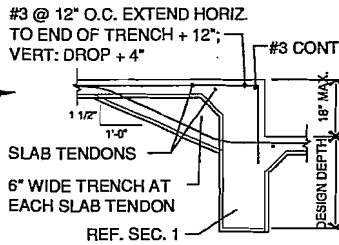
(1C) - HEIGHTENED CURB - (1D)



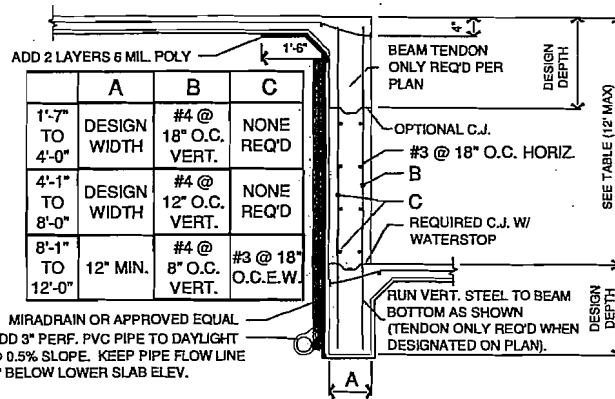
(1A) - STANDARD CURB - (1B)



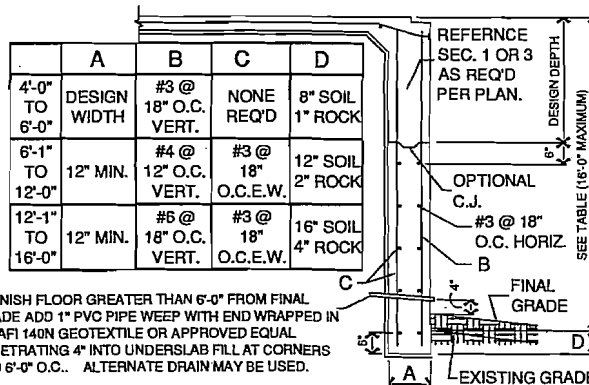
(E) - EXTENDED BRICK LEDGE - (F)



DROP IN SLAB 7 TO 18 INCHES - (5)



DROP IN SLAB 19 INCHES TO 12 FEET - (6)

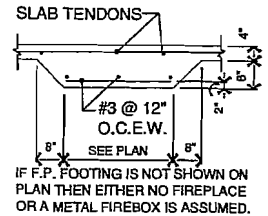


IF FINISH FLOOR GREATER THAN 6'-0" FROM FINAL GRADE ADD 1" PVC PIPE WEEP WITH END WRAPPED IN MIRAFI 140N GEOTEXTILE OR APPROVED EQUAL PENETRATING 4" INTO UNDERSLAB FILL AT CORNERS AND 6'-0" O.C.. ALTERNATE DRAIN MAY BE USED.

DEEP BEAM 4 TO 16 FEET

DEEP BEAM AND SECTION 6 NOTES:

- FORMING FILL BEHIND DROP IN SLAB OR DEEP BEAM OVER 4 FEET HIGH TO BE RETAINED BY 16" WIDE FILL BAGS OF WOVEN PLASTIC AND FILLED WITH CRUSHED STONE OR WASHED GRAVEL. FOR HEIGHTS OVER 8 FEET, USE TWO ROWS OF BAGS FULL HEIGHT.
- STIRRUPS MAY BE USED I.L.O. VERT. REINFORCING, SIZE AND SPACING PER "C" CATEGORY ON CHART.



INT. F.P. FTG. - (7)

1-0. GENERAL

- Inspections by Engineer for compaction; tendon placement; testing; tendon stressing; pier ins.
- Tendon lengths and count and purposes only. Contractor's installation. Concrete quantities are not to be used.
- Plan shows the location of structural members. Architectural dimensions must be used. Report any discrepancies on architectural plans—not the structural plans.
- The latest B.F.A.B. and/or S.F.A.B. shall be used for establishing the design requirements. Refer to the Building Code, the Southern Standard Building Code, the Southern Standard Dwelling Code, latest adopted.
- These plans are copyright © 1998.
- Vertical control joints shall be spaced approximately 25 feet apart. A maximum of 2 joints per slab.
- The MLAW Installation Guide, 2nd Edition, (The Guide) shall be used for full length.

2-0. SITE PREPARATION

- All site work shall be performed in accordance with the plans. Refer to notes concerning "Approvals".
- All underslab "Forming Fill" shall be in accordance with the plans.

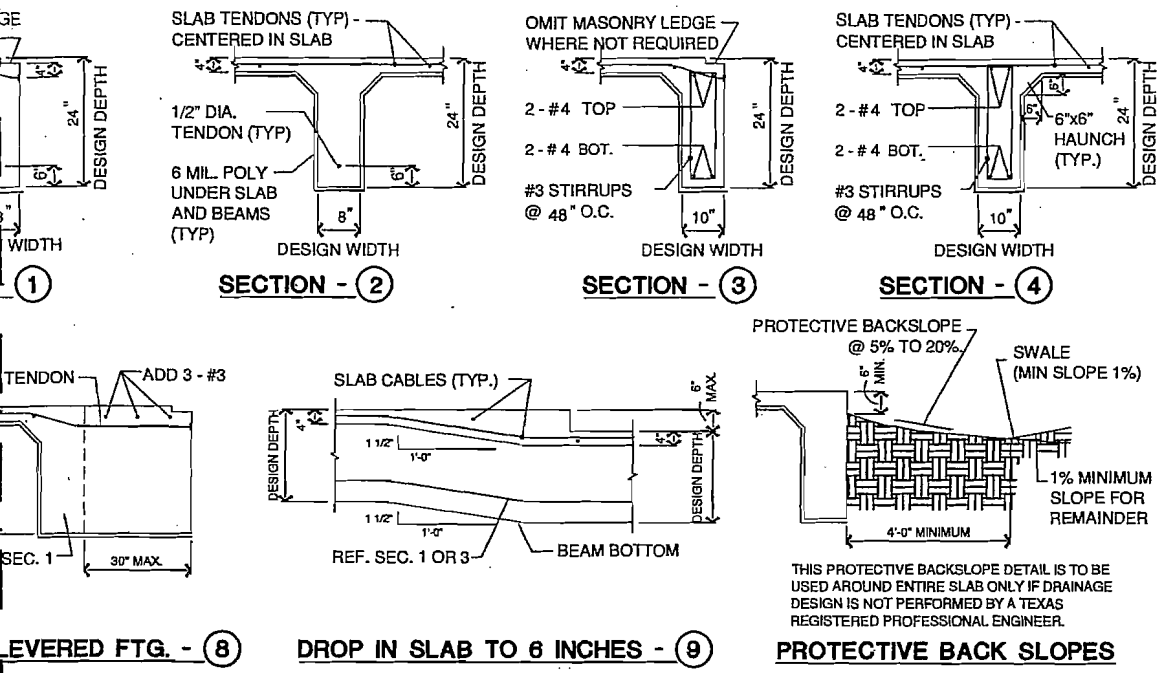
3-0. CONCRETE

- Concrete shall have a minimum strength of 2000 psi at 28 days. Concrete shall be deposited in layers not exceeding 18 inches. One addition of water will be permitted.
- Concrete shall be well consolidated. Consolidation shall be done in the vicinity of the tendon anchorage.
- If conduit in slab is required, specify size, location, and depth. Piping, vents or electrical conduits and/or conduits larger than 2 inches shall be specified.
- If unanticipated interruptions occur, temporary forms must be used to form vertical joints prior to casting into existing concrete to be placed.
- FLATWORK MAY BE PLACED**

4-0. CONCRETE COVERAGE

- SLAB TENDONS:**
 - 1-1/2 inches above subgrade.
 - 1-1/2 inches above vertical coverage from concrete.
- Slab Tendons may be modified. Beam Tendons may be modified.
- Beam and Wall Steel shall be 1-1/2 inches slab, 2 inches tendon.
- PIPE PENETRATIONS:**
 - 2 inches for tendon and 1-1/2 inches for pipe.

EXHIBIT 3



like Foundation People

2804 LONGHORN BLVD.
AUSTIN, TEXAS 78758
(512) 835-7000

THE SEAL APPEARING ON THIS DOCUMENT WAS AUTHORIZED BY CHRISTOPHER S. COPELAND, P.E. 89672 ON 10-19-04

NOTES

...entative required, as applicable, for: fill placement and ... of beams and slab; concrete placement and ... n. ... ete quantity estimate on plan are for estimating ... all tendon lengths and concrete quantity prior to ... adjusted for sloping site and forming irregularities. ... ed tendons are not shown, U.N.O., for plan clarity. ... reinforcement, beam depth and beam locations only. ... pared to the architectural plans prior to construction of ... Engineer. The forms should be built using the ... plan. Do not scale plan. ... a along with the latest A.C.I. Code have been used in ... of this foundation. Foundations constructed in ... ications will meet the requirements of the Uniform ... Building Code and the CABO One And Two Family ... & Standards Of Good Engineering Practice. ... of the year dated. ... in exterior masonry to the full height spaced ... could be located directly above all slab control joints. ... e specifications for Slab-On-Ground Foundations, latest ... ed into these plans by reference as if recited herein at ...

... accordance with FHA Data Sheet 79-G or The Guide. ... and "unapproved" fill. ... a P.I. less than 20 and be free of organics. ...

... compressive strength of 3000 psi at 28 days. Concrete ... on stressing. All concrete work shall meet A.C.I. 318. ... later than two hours after water is mixed at the plant. ... t the job site to adjust the slump to a maximum of 6 ...

... using proper mechanical vibration, especially in the ...

... concrete placement, location to be verified in field. ... e placed so as not to reduce slab thickness. Plumbing ... must be trench into underslab fill. ... ete placement occur, and concrete hardens, ... ng of construction joints or concrete must be chipped ... tional slab. Use #3 X 24" dowels at 12" O.C. epoxied ... concrete. ... AFTER STRESSING. ...

... thick slab and ANCHORS to have 4 inches ... or to top of concrete. ... X. horizontally to allow for plumbing box-outs. ... nward and/or 2" upward vertically for ...

... nd 3 inches exposed to earth.

- 5-0. REINFORCING**
- 5-1. All reinforcing bars shall be ASTM A-615 Grade 60, except Grade 40 may be used for stirrups, corner bars and hairpins.
 - 5-2. All tendons shall be 270k grade, 7 wire strand, 1/2 inch diameter, U.N.O., greased and sheathed with a continuous extruded plastic sheathing.
 - 5-3. Anchorage system shall be a monostrand unbonded tendon anchorage utilizing a cast wedge plate and a two piece wedge as manufactured by a P.T.I. approved manufacturer.
 - 5-4. All post-tensioned tendons and anchors shall conform to the requirements of the latest "P.T.I. Guide Specifications For Post-Tensioning Materials." Post-tensioned tendon supplier to be P.T.I. factory certified.
 - 5-5. **PARTIAL STRESS** all tendons to 13.3 kips (or half of final jacking force) 24 to 48 hours after concrete placement.
 - 5-6. **FULL STRESSING** of all tendons to 33 kips 7 to 10 days after concrete placement.
 - 5-7. The first tendon in the slab shall be a maximum of 14 inches and a minimum of 6 inches from the outside form. Tendons not dimensioned on plan to be equally spaced.
 - 5-8. (1) #3 x 24 inches x 24 inches corner bar required at all exterior corner's top, U.N.O. Deepened beams to have corner bars with diameter equal to horizontal steel at each horizontal bar.
 - 5-9. At plumbing stacks, add #3 bars x size of opening plus 16 inches to be placed in concrete 2 inches beyond perimeter of opening.
- 6-0. PLAN VARIATIONS**
- 6-1. All depth dimensions of beams are minimum unless intact rock is encountered at less depth. Inspector may approve beams continuously on rock to minimum beam depth of 12 inches. Deepen beams where required by site conditions at least 6 inches into virgin soil, or to rock unless deep beam detail applies.
 - 6-2. When PI is 38 and greater and trees are within 15 feet of foundation, consult MLaw "Policies Concerning Trees" latest revision.
 - 6-3. Should conditions arise that are not covered by details on this plan, contact Engineer at once for additional instructions.
 - 6-4. In areas to receive tile, we recommend installing 6x6x1.4x1.4 WWF 1-1/2" below concrete surface and bedding the tile on a bond breaker to prevent shrinkage cracks from reflecting through the tile.
 - 6-5. **HARD POINTS** - If the depth of underslab clean fill at any beam intersection exceeds 48 inches, place hard points through the fill. Use of 12 inch diameter pre-formed or drilled, concrete piers OR 18 inch by 18 inch square footings is acceptable. And all beams to have tendons or steel. (If hardpoint depth exceeds 6'-0" from top of slab reinforce w/ (4)-#4 vert. & #3 ties @ 24" O.C.) If total underslab fill exceeds 12 feet, contact Engineer.
- 7-0. TREE POLICY- APPLIES TO P.I.'S = 38 AND GREATER**
- 7-1. **TREE WITHIN 5 FEET FROM FOUNDATION:**
 - a. Add 20'-0" of section 3 steel - center on tree in exterior beam only, OR
 - b. Deepen beam 24" into existing soil for 20'-0" - exterior beam only.
 - 7-2. **TREE 5 TO 15 FEET FROM FOUNDATION:**
 - a. Add 20'-0" of section 3 steel - center on tree in exterior beam only, OR
 - b. Deepen beam 12" into existing soil for 20'-0" - exterior beam only.
 - 7-3. Add 6" wide trench 24" into existing grade 20'-0" long centered on tree and filled with un-reinforced concrete.
- OPTIONAL PROVISIONS TO BE ENFORCED, IF CHECKED:**
- FILL (UNAPPROVED).** The fill material on this site is unsuitable to support a slab-on-ground foundation. The fill must be penetrated by all grade beams and extend a minimum of 6 inches into virgin soil. As an alternative, see **HARD POINTS** note. Based on the soils investigation, unapproved fill appears to be approximately deep.
 - FILL (APPROVED).** The fill material is acceptable to support a slab-on-ground foundation. Construct exterior grade beams 6 inches into approved fill. *Approved Fill* is fill that has been approved by MLaw, based on proper exploration, testing, or inspection by an agency acceptable to MLaw.

SOIL DATA

PTI SOIL PARAMETERS

	CENTER	EDGE
EM:	5.5	3.4
YM:	2.3	0.8

BEARING CAPACITY: 2000 PSF

SOURCE: MLA LABS

DATE: 8-2004

DESIGN P.I.: 32FS

CAPITAL PACIFIC HOMES

Job No: 4611001.036

240 CANTERBURY DRIVE
BELTERRA
City: DRIPPING SPRINGS

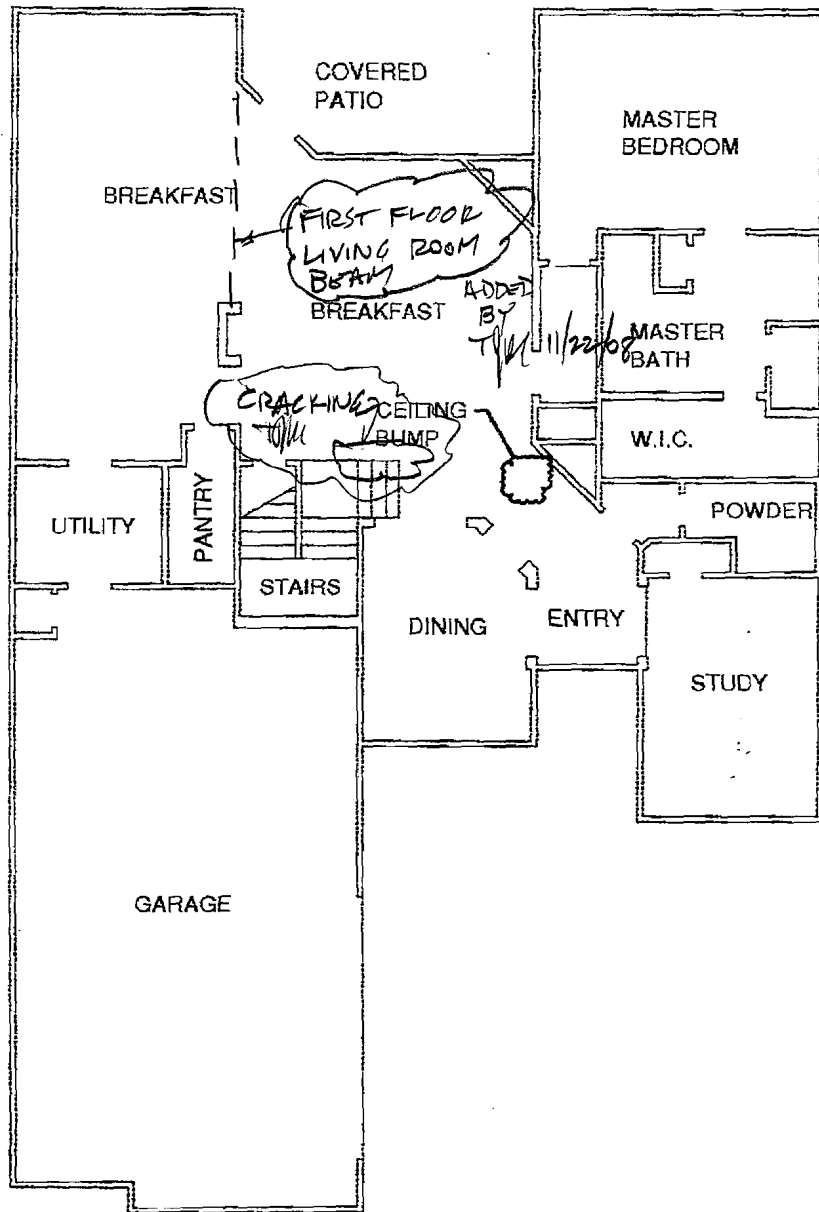
Section: 2 Phase: 2
Lot: 15 Block: C
Plan #: 2919 DLSE
Date: 10/19/2004 Drawn: JJK
Revision: Check:

STRESSING CHART

ELONGATION VALUES WITH PAINTMARK

CABLE LENGTH	ELONGATION	CABLE LENGTH	ELONGATION
15'-18"	4 1/2"	79'-84"	10"
19'-24"	5"	85'-90"	10 1/2"
25'-30"	5 1/2"	91'-96"	11"
31'-35"	6"	97'-103"	11 1/2"
35'-41"	6 1/2"	104'-109"	12"
42'-47"	7"	110'-115"	12 1/2"
48'-53"	7 1/2"	115'-122"	13"
54'-59"	8"	123'-128"	13 1/2"
60'-65"	8 1/2"	129'-135"	14"
66'-71"	9"	135'-142"	14 1/2"
72'-78"	9 1/2"	143'-148"	15"

EXHIBIT 4



LOWER LEVEL WALLS

READINGS IN .25 INCH INCREMENTS

ADDRESS: 240 CANTERBURY DRIVE
 JOB No: 714008000.004

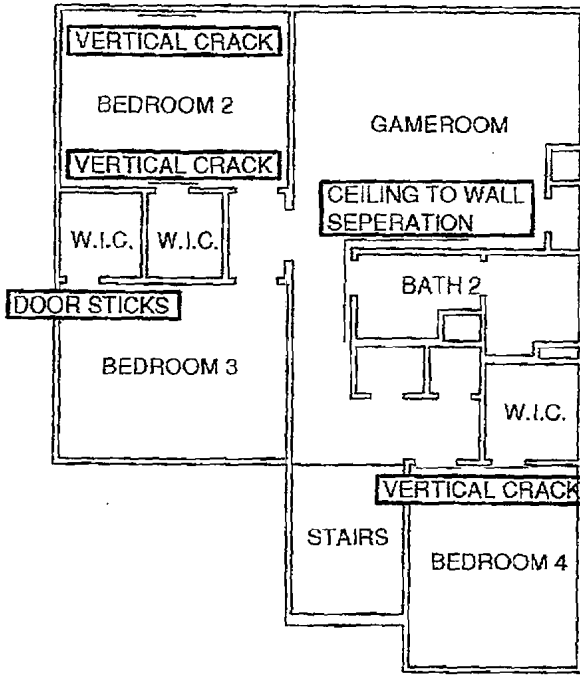
CLIENT: CAPITAL PACIFIC
 DATE: 4-25-07
 SHEET:

GEOSTRUCTURAL/STRUCTURAL
 GEOTECHNICAL/PAVEMENTS
 FORENSIC



2804 LONGHORN BLVD.
 AUSTIN, TEXAS 78756
 (512) 835-7000

EXHIBIT 5

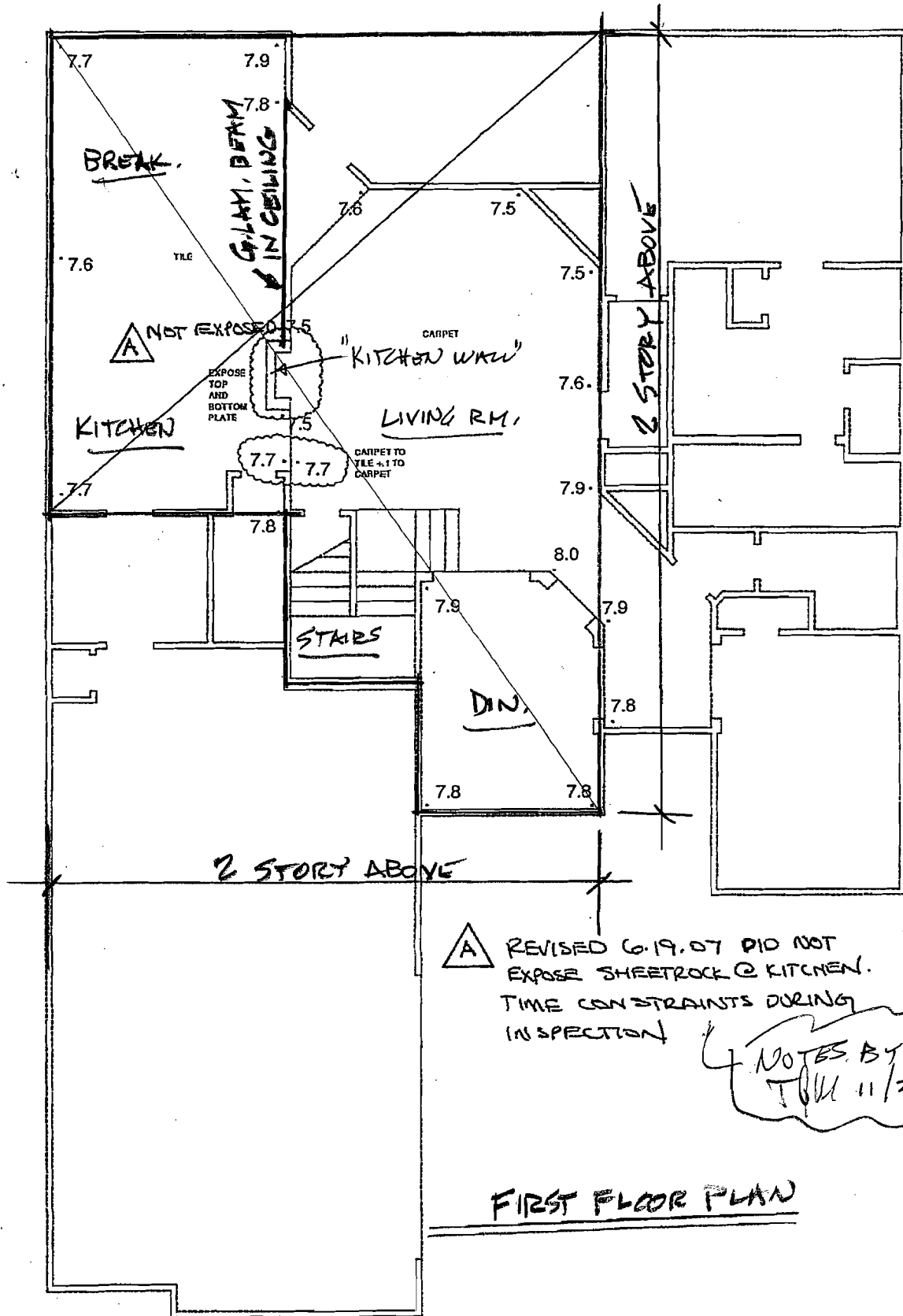


UPPER LEVEL WALLS

READINGS IN .25 INCH INCREMENTS

<p>ADDRESS: 240 CANTERBURY DRIVE JOB No: 714008000.004</p>	<p>CLIENT: CAPITAL PACIFIC DATE: 4-25-07 SHEET:</p>	
<p>GEOSTRUCTURAL/STRUCTURAL GEOTECHNICAL/PAVEMENTS FORENSIC</p>	<p>MLAW CONSULTANTS AND ENGINEERS</p>	<p>2804 LONGHORN BLVD. AUSTIN, TEXAS 78758 (512) 835-7000</p>

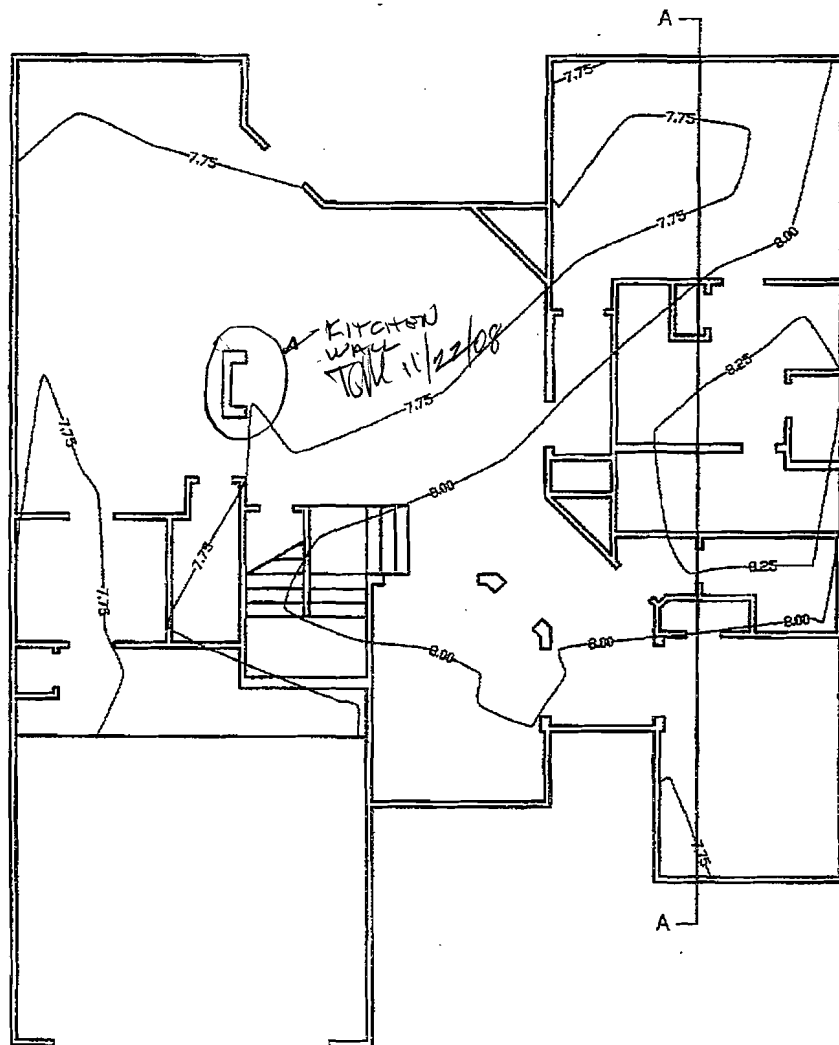
EXHIBIT 6



A REVISED 6-19-07 DID NOT EXPOSE SHEETROCK @ KITCHEN. TIME CONSTRAINTS DURING INSPECTION

NOTES BY OTHERS
 TJK 11/22/08

FIRST FLOOR PLAN



SLAB SURFACE CONTOURS

READINGS IN .25 INCH INCREMENTS

ADDRESS: 240 CANTERBURY DRIVE
 JOB No: 714008000.004

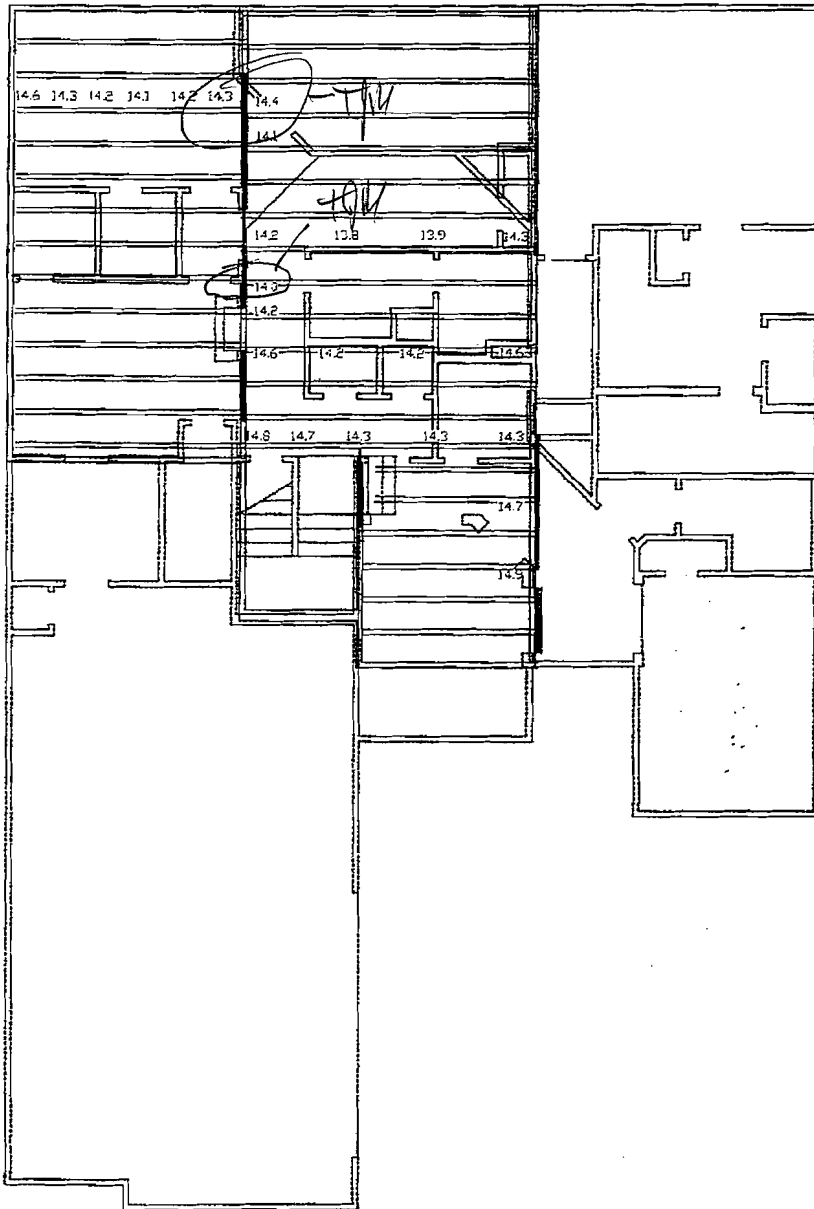
CLIENT: CAPITAL PACIFIC
 DATE: 4-25-07
 SHEET: 1 OF 3

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EXHIBIT 8



LOWER LEVEL CEILING ELEVATIONS

READINGS IN .25 INCH INCREMENTS

ADDRESS: 240 CANTERBURY DRIVE
 JOB No: 714008000.004

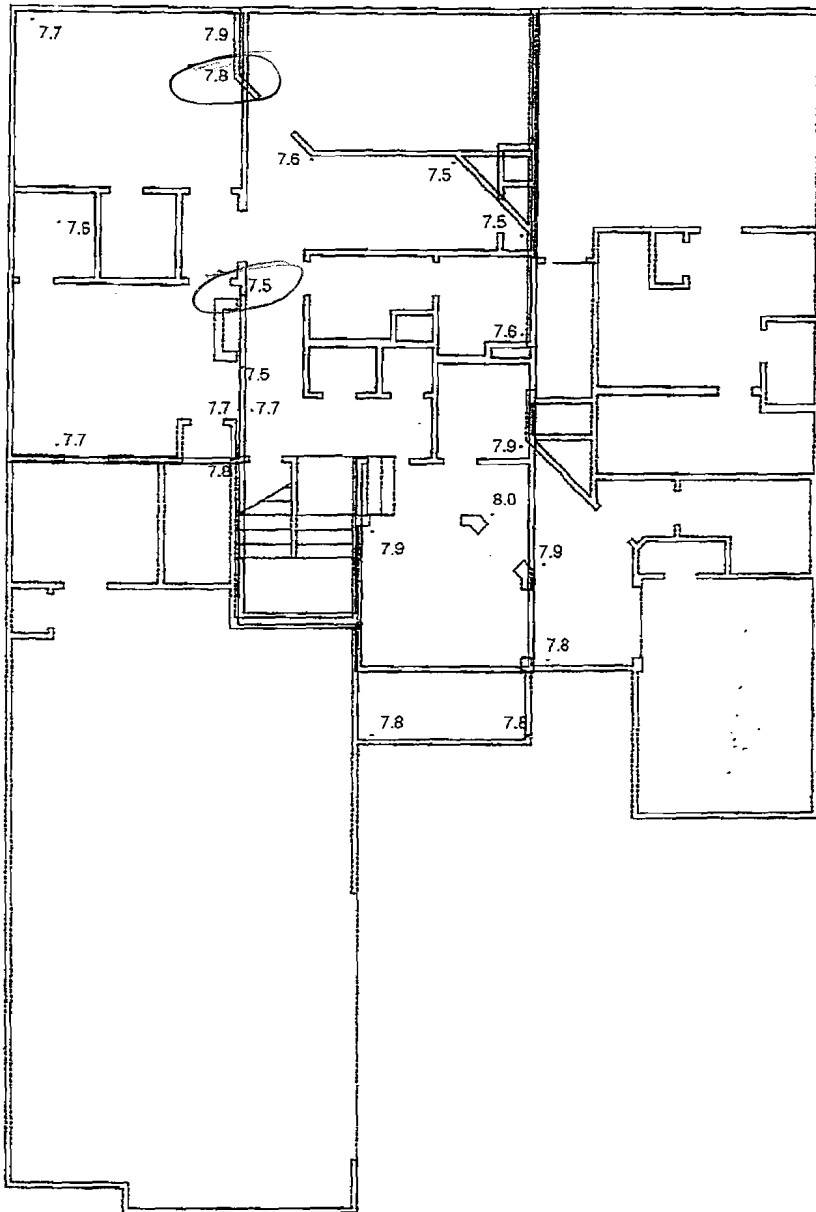
CLIENT: CAPITAL PACIFIC
 DATE: 4-25-07
 SHEET:

GEOSTRUCTURAL/STRUCTURAL
 GEOTECHNICAL/PAVEMENTS
 FORENSIC



2804 LONGHORN BLVD.
 AUSTIN, TEXAS 78758
 (512) 835-7000

EXHIBIT 9



**FOUNDATION ELEVATION READINGS WITH 2nd
LEVEL OVERLAY**

READINGS IN .25 INCH INCREMENTS

ADDRESS: 240 CANTERBURY DRIVE
JOB No: 714008000.004

CLIENT: CAPITAL PACIFIC
DATE: 4-25-07
SHEET:

GEOSTRUCTURAL/STRUCTURAL
GEOTECHNICAL/PAVEMENTS
FORENSIC



2804 LONGHORN BLVD.
AUSTIN, TEXAS 78768
(512) 835-7000

EXHIBIT 10

56-2 X 4 X 4' SYP
 8-2 X 8 X 14' SYP
 5-2 X 8 X 16'

HANGERS:

1-HGUS 410
 26-HUS 26
 1-THA 413

*TRUSSES
 OVER
 2-STORY*

GIRDER T56&T6G,T14G
 PLIES 2
 NAILING SCHEDULE (10d_box_parts)
 TOP ROW 12 O.C
 BOT 12
 WEB 4

THE FOLLOWING TRUSSES REQUIRE
 SCABS. REFER TO ENGINEERING FOR
 NAILING SCHEDULES

T15G, T16G T5G, T6G,

CAPITAL PACIFIC HOMES
 PLAN # 2919

ELEVATION D
 RAISED ENTRY
 CANTILEVER BACK
 EXT GAR

2' POP UP

BUFFALO FRAMING
 1510 THREE POINTS
 PFLUGERVILLE, TX. 78660
 (512)670-2300

DESIGNED BY SG

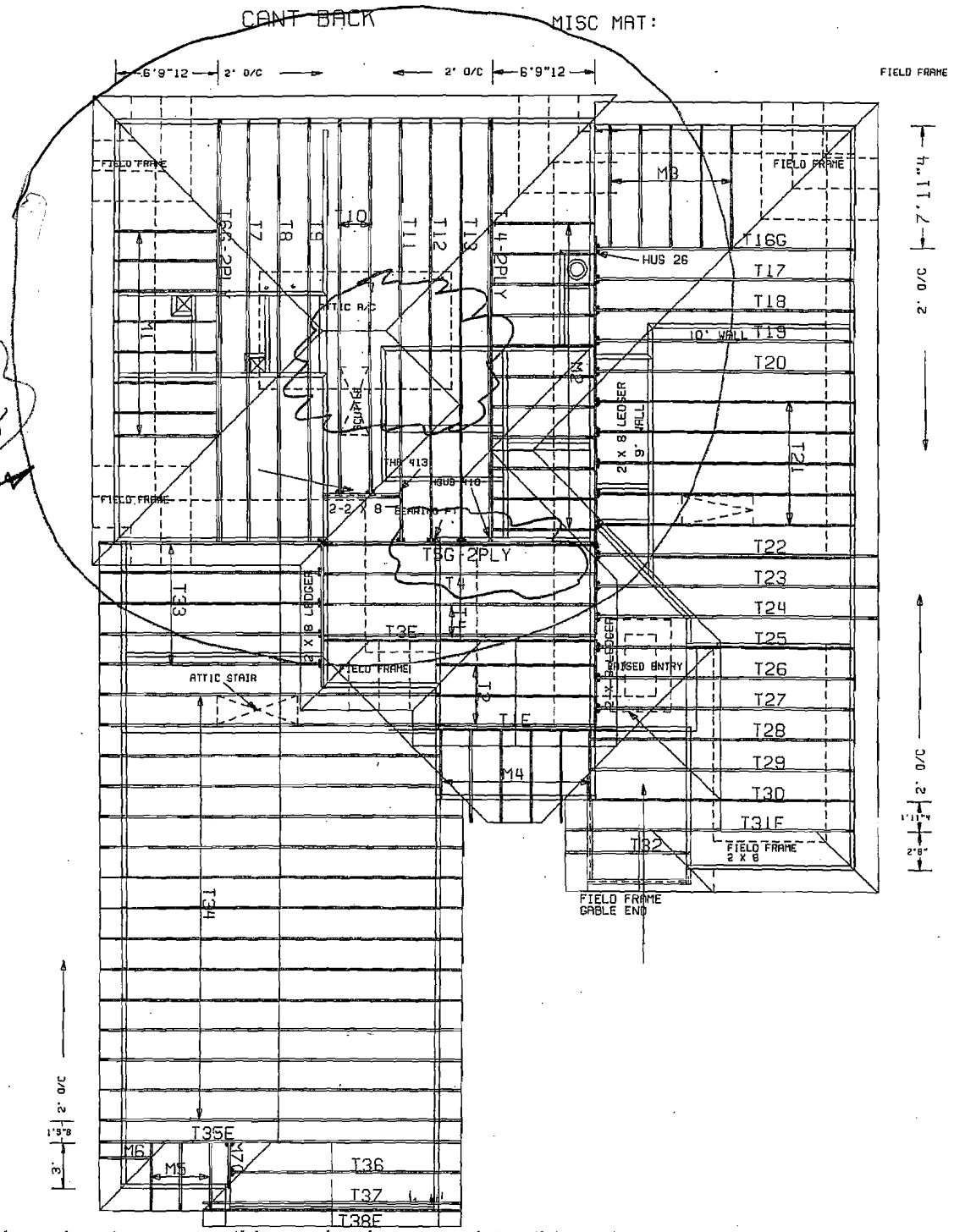


EXHIBIT 11

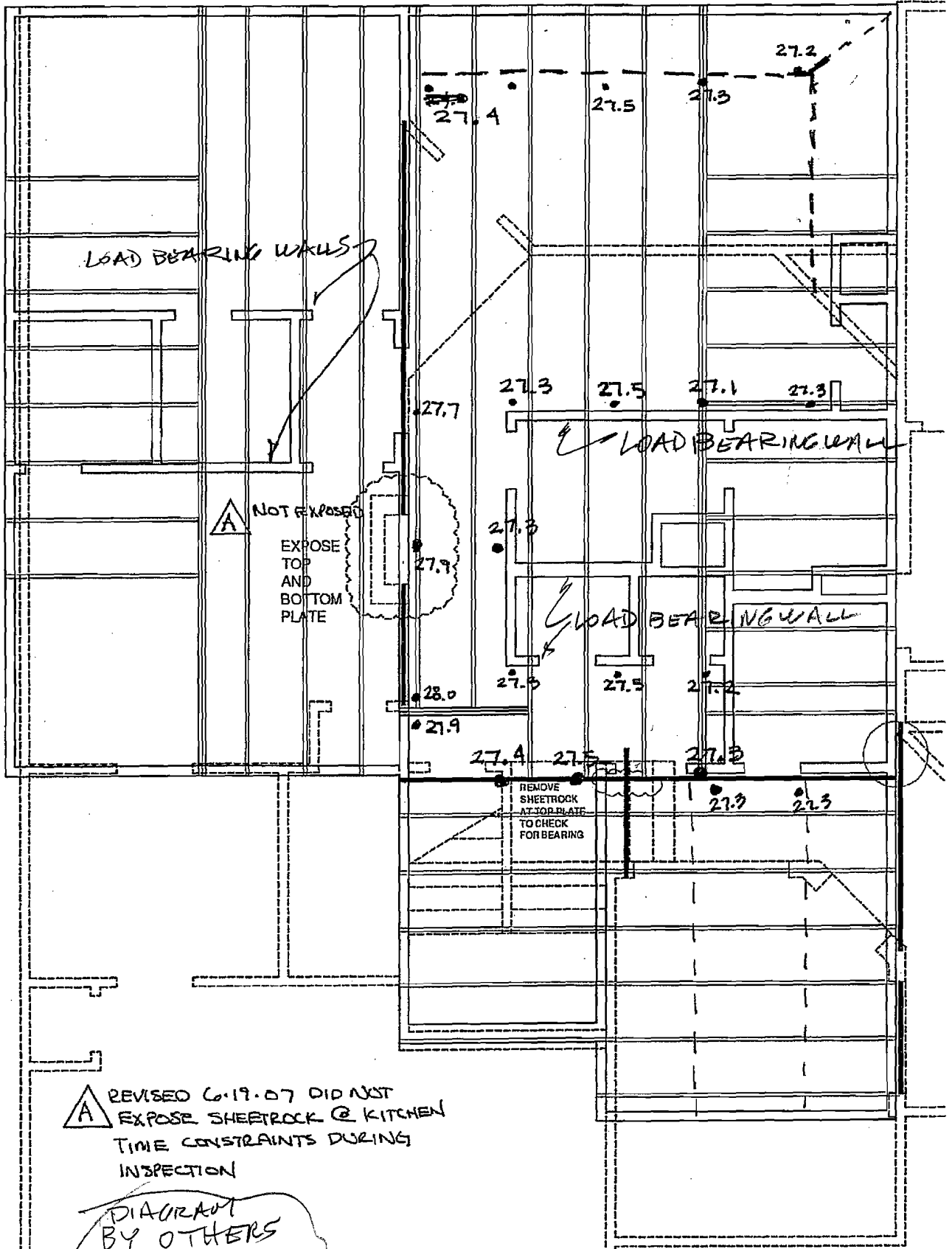
JOB LOCATION:
 240 CANTERBURY

JOB DESCRIPTION:
 KWOOD D / CANT REAR / SIDE E

DESIGNED BY:
 STAN GILLESPIE

JOB NO:
 C2919DAS

PAGE NO:
 1 OF 1



A REVISED 6.19.07 DID NOT
 EXPOSE SHEETROCK @ KITCHEN
 TIME CONSTRAINTS DURING
 INSPECTION

DIAGRAM
 BY OTHERS
 TQM 11/22/08

EXHIBIT 12

5.11.07

(C29190AS-KW000 0 / CANT REAR / SIDE ENT / AT A/C -- 240 CANTERBURY - 711)

Top chord 2x4 SP #2 :T1 2x8 SP #2:
 Bot chord 2x6 SP #2
 Webs 2x4 SP #3 :W15 2x4 SP #2:

80 mph wind, 23.39 ft mean hgt, ASCE 7-93, CLOSED bldg, not located within 4.50 ft from roof edge, 100 mi from coast, CAT I, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
 TC - From 56 PLF at 0.00 to 56 PLF at 27.63
 PLT - From 4 PLF at 12.81 to 4 PLF at 14.81
 BC - From 14 PLF at 0.00 to 14 PLF at 10.52
 BC - From 54 PLF at 10.52 to 54 PLF at 17.11
 BC - From 14 PLF at 17.11 to 14 PLF at 27.63
 BC - 23 LB Conc. Load at 10.52, 17.11
 BC - 855 LB Conc. Load at 24.63

Calculated horizontal deflection is 0.14" due to live load and 0.15" due to dead load.

H - recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/480 live and L/360 total load.

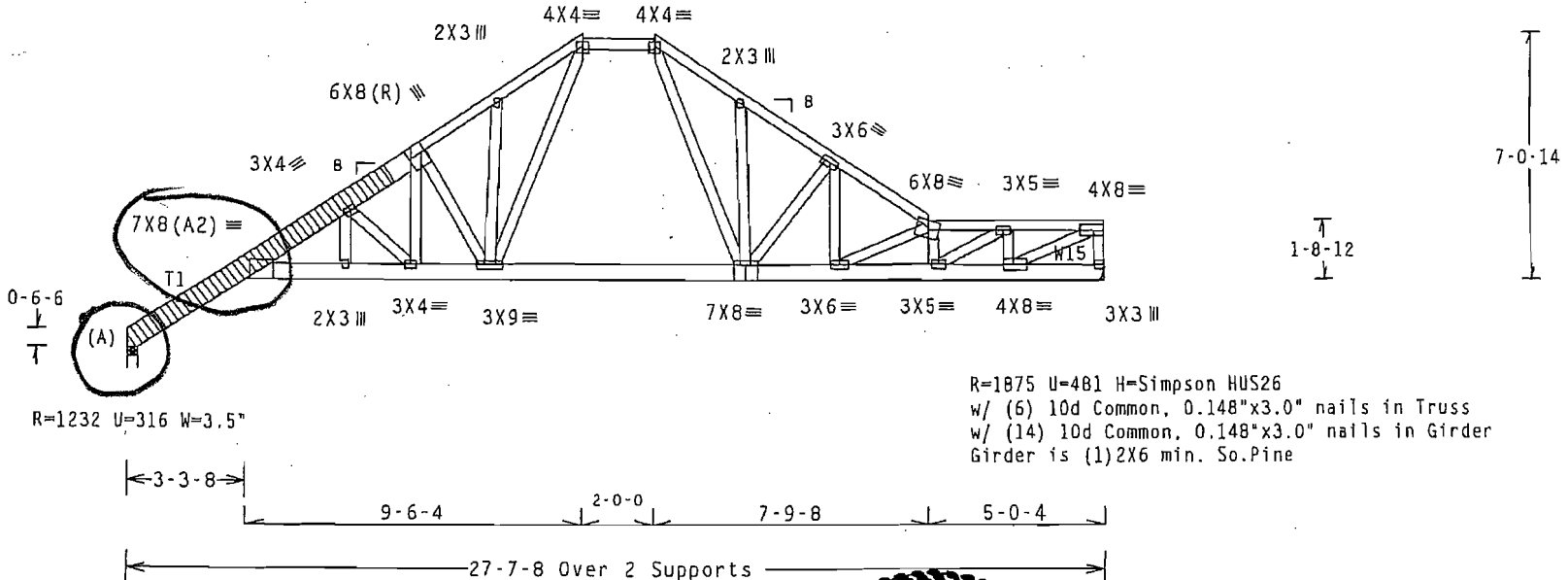
(2) 2x8x8-10-15 SP #2 scabs at left end. Attach one scab to each outer face of chord with: 10d_box_nails @ 8" OC, Plus additional nail clusters at: BRG.: (4), heel: (9), 1st panel point: (5).

016

BUFFALO FRAMING

05/14/07 09:18 FAX 512 670 2301

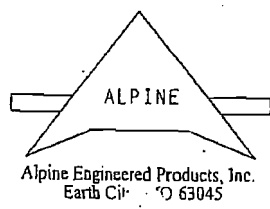
EXHIBIT 13



PLT TYP. WAVE TPI\R

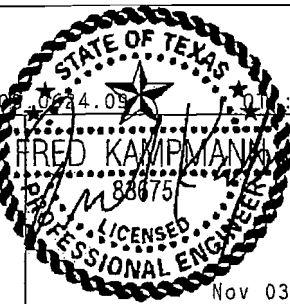
Design Crit: TPI-1995(STD)/UBC

7.00 0624.09 1 TX/-/1/-/R/- Scale =.2"/Ft.



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND DRACING. REFER TO BCSP 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 363 D'ORFALO DR., SUITE 200, MADISON, WI 53729) AND NCTCA (NORTH TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE BL, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. OF STEEL) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H,U,S,K) ASTM A500 GRADE 40/50 (H, K,U,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC. 7. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R6918- 65370
TC DL	8.0 PSF	DATE 11/03/04
BC DL	7.0 PSF	DRW MOUSR6918 04308011
BC LL	0.0 PSF	MO-ENG /FK
TOT.LD.	35.0 PSF	SEQN- 23718
DUR.FAC.	1.25	
SPACING	24.0"	

(C2919DAS-KWOOD D / CANT REAR / SIDE ENT / AT A/C -- 240 CANTERBURY - 712)

Top chord 2x4 SP #2 :T1 2x8 SP #2:
 Bot chord 2x6 SP #2
 Webs 2x4 SP #3

80 mph wind, 22.72 ft mean hgt, ASCE 7-93, CLOSED bldg, not located within 4.50 ft from roof edge, 100 mi from coast, CAT I, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.15" due to live load and 0.14" due to dead load.

Right end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

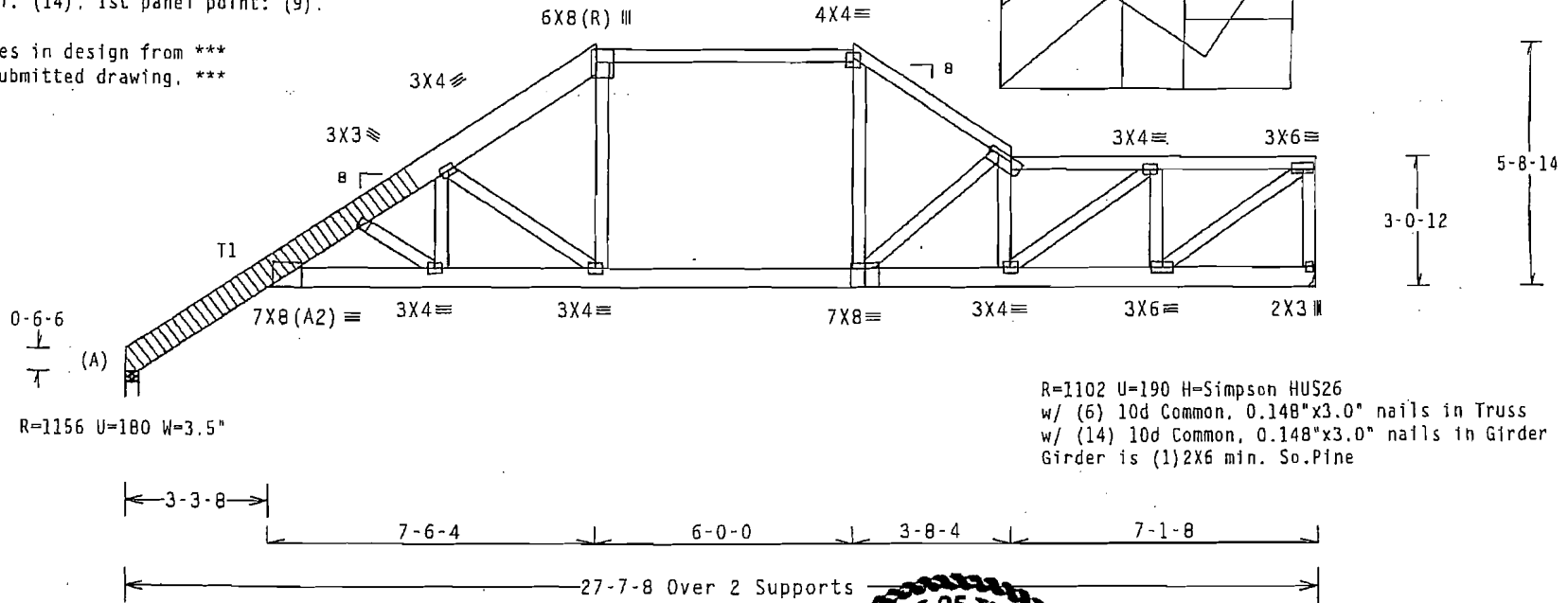
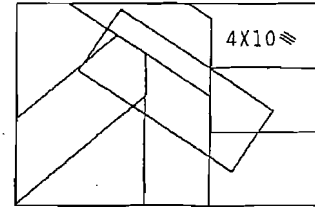
BC attic room floor loading: LL = 15.00 psf; DL = 5.00 psf; from 11-1-4 to 16-9-12.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/480 live and L/360 total load.

(1) 2x8x8-0-1 SP #2 scab at left end. Attach scab to face of chord with: 10d_box_nails @ 8" OC, plus additional nail clusters at: BRG.: (6), heel: (14), 1st panel point: (9).

*** Note changes in design from ***
 *** original submitted drawing. ***



R=1102 U=190 H=Simpson HUS26
 w/ (6) 10d Common, 0.148"x3.0" nails in Truss
 w/ (14) 10d Common, 0.148"x3.0" nails in Girder
 Girder is (1)2X6 min. So.Pine

EXHIBIT 14

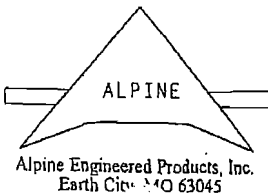
PLT TYP. WAVE TPI\R

Design Crit: TPI-1995(STD)/UBC

7.00 R624.00

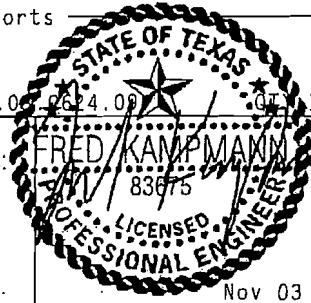
TX/-/1/-/R/-

Scale =.25"/Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 502 D'ORFORD DR., SUITE 200, MADISON, WI 53719) AND WEA (WOOD ENGINEERING ASSOCIATION OF AMERICA, 8300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO CONFORMANCE WITH TPI OR FABRICATION, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTION PLATES ARE MADE OF 20/18/10GA (M,N,S,X) ASTM A575 GRADE 40/50 (W, F,H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 10A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANHX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS1/FP1 3 SEC. 2.

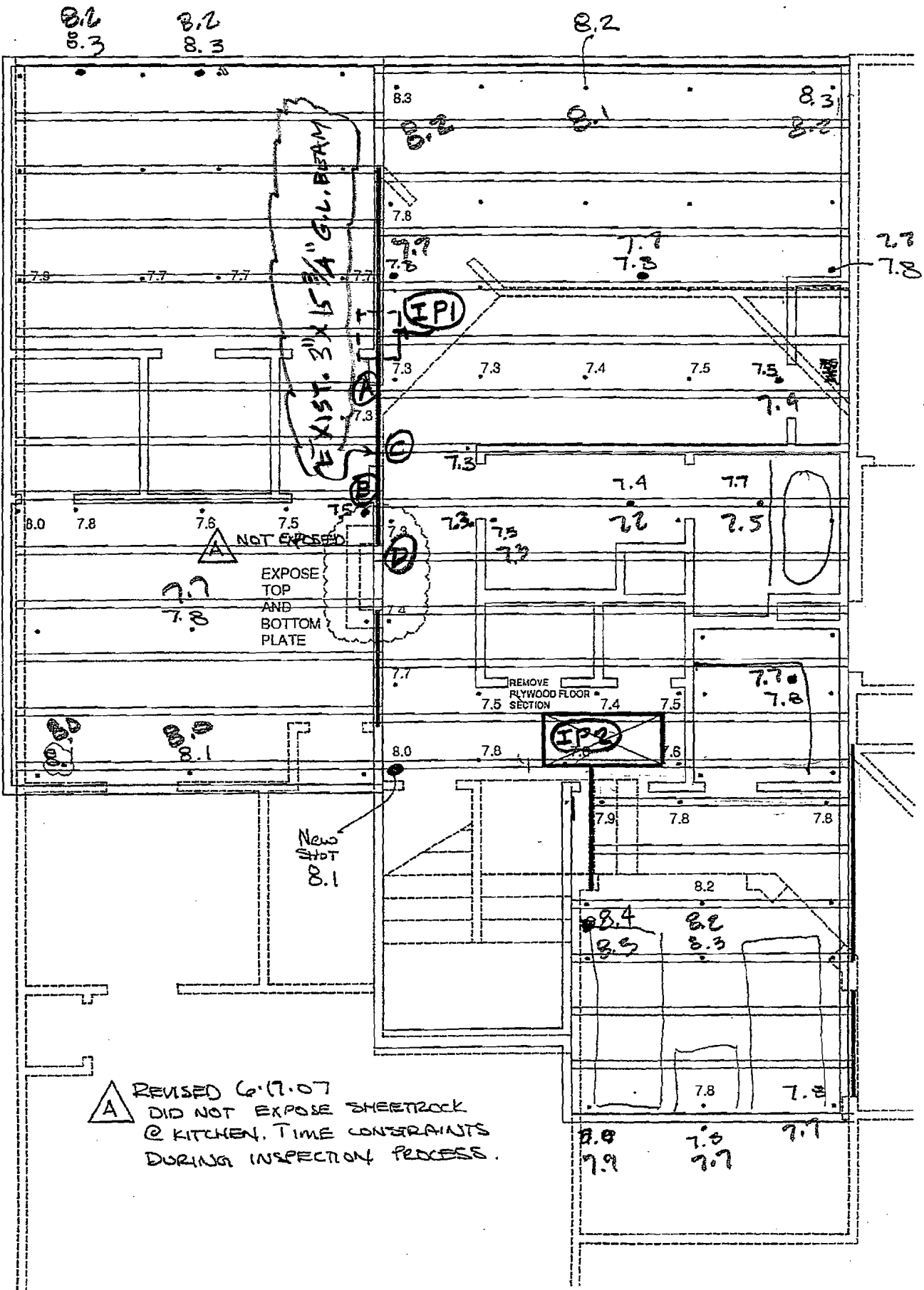


TC LL	20.0 PSF	REF R6918- 65371
TC DL	8.0 PSF	DATE 11/03/04
BC DL	7.0 PSF	DRW MOUSR691B 04308036
BC LL	0.0 PSF	MO-ENG /FK
TOT.LD.	35.0 PSF	SEQN- 23715
DUR.FAC.	1.25	
SPACING	24.0"	

017

BUFFALO FRAMING

05/14/07 09:18 FAX 512 670 2301



A REVISED 6.17.07
 DID NOT EXPOSE SHEETROCK
 @ KITCHEN. TIME CONSTRAINTS
 DURING INSPECTION PROCESS.

EXHIBIT 15

EXHIBIT 1C

MISC MAT:

- 1-3 1/8 X 16 X 16' GLB
- 1-3 1/8 X 12 3/8 X 10' GLB

- 2-2 X 12 X 12' SYP
- 1-2 X 12 X 10' SYP

OR GREATER

HANGERS:
23-THA 413

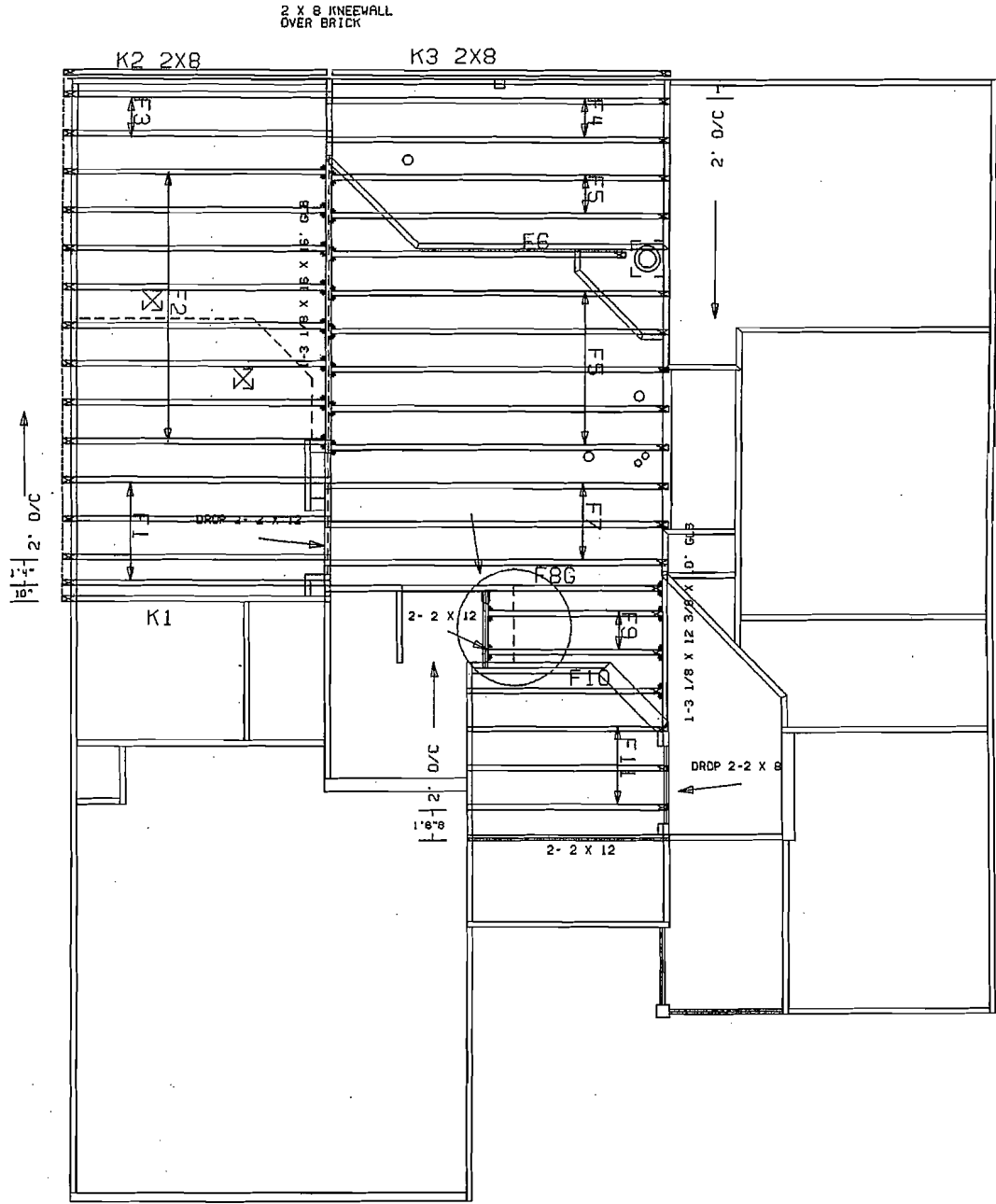
SPACE TRUSS FOR PLUMBING
AS REQUIRED (NOT TO EXCEED
4")

2X6 STRONG BACKS REQUIRED ON
TRUSSES EXCEEDING 10'0" SPAN

CAPITAL PACIFIC HOMES
PLAN # 2919

REV FLOORS
OPT. MUD RM

BUFFALO FRAMING
1510 THREE POINTS
PFLUGERVILLE, TX. 78660
(512) 670-2300



JOB LOCATION:
3948 LORD BYRON CR

JOB DESCRIPTION:
KWOOD / MUD RM / CANT SIDE &

DESIGNED BY:
STAN GILLESPIE

JOB NO:
C2919FCR

PAGE NO:
1 OF 1

THA/THAC Adjustable Truss Hangers



✓ This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

The THA series' extra long straps allow full code nailing and can be field-formed to give top flange hanger convenience.

Designed for 4x2 trusses and 4x beams, the THAR/L422 has a standard skew of 45 degrees. Straps can be formed to give top flange hanger convenience. PAN nailing helps eliminate splitting of 4x2 truss bottom chords.

MATERIAL: See table.

FINISH: Galvanized. Some products available in Z-MAX; see Corrosion-Resistance, page 6-7.

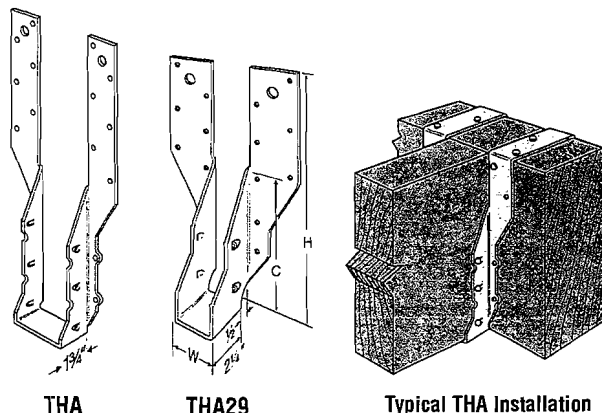
INSTALLATION: • Use all specified fasteners. See General Notes.

Two different installation methods may be used:

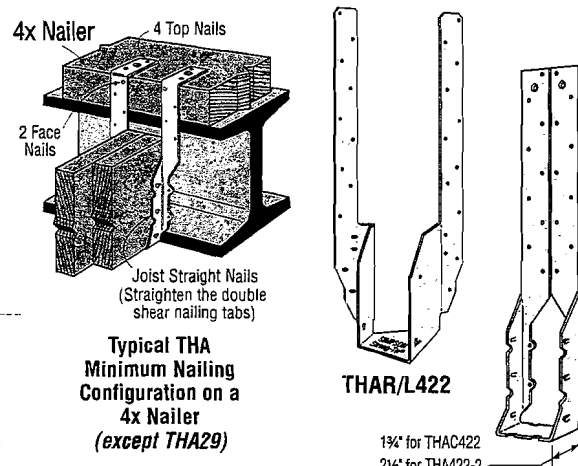
Maximum nailing—Install all face nails according to the table. Nails used for the joist attachment must be driven at an angle so that they penetrate through the corner of the joist into the header. With single 2x carrying members, use 10dx1½" nails into the carrying member, and 10d or 16d commons into the carried member, and use 0.81 of the table value for 18 gauge, and 0.68 of the table value for 16 gauge.

Minimum nailing—For the THA29, the minimum nailing schedule requires the use of joist double shear nailing as detailed above, and that the strap be field-formed over the header a minimum of 2½". A minimum of four top and four face nails must be used. For all models except the THA29, the minimum nailing schedule may be followed where double shear nailing is not possible, provided the strap is field-formed over the top of the header a minimum of 1½" for the THA213 and THA413, and 2" for all others, and a minimum of four top and two face nails are used.

CODES: See page 12 for Code Listing Key Chart.



THA THA29 Typical THA Installation

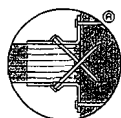


Typical THA Minimum Nailing Configuration on a 4x Nailer (except THA29)

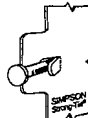
THAR/L422

1½" for THA422
2½" for THA422-2 and THA426-2

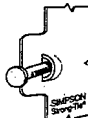
THA422



Double Shear Nailing Top View



Double Shear Nailing Side View



Dome Double Shear Nailing Side View (available on some models)
U.S. Patent 5,603,580

Available with additional corrosion protection. Check with factory.

Minimum Carried Member	Model No.	Ga	Dimensions			Fasteners ¹				Doug-Fir-Larch/Southern Pine Allowable Loads ⁴				Spruce-Pine-Fir Allowable Loads ⁴			Code Ref.	
			W	H	C	Carrying Member		Carried Member		Uplift ² (133 & 160)	Floor (100)	Snow (115)	Roof (125)	Uplift ² (133 & 160)	Floor (100)	Snow (115)		Roof (125)
						Top	Face	Straight	Slant									
MINIMUM NAILING — TOP FLANGE																		
2x4	THA29	18	1½"	9¼"	5½"	4-10d	4-10d	—	4-10d	560	2260	2310	2350	480	1740	1785	1815	1, 84, 140
2x6	THA213	18	1½"	13¾"	5½"	4-10d	2-10d	4-10dx1½"	—	—	1615	1615	1615	—	1280	1280	1280	1, 84, 121, 140
2x6	THA218	18	1½"	17¾"	5½"	4-10d	2-10d	4-10dx1½"	—	—	1615	1615	1615	—	1280	1280	1280	1, 84, 121, 140
(2)2x10	THA218-2	16	3½"	17¼"	8"	4-16d	2-16d	6-16dx2½"	—	—	1635	1635	1635	—	1465	1465	1465	1, 84, 121, 129, 140
(2)2x10	THA222-2	16	3½"	22¾"	8"	4-16d	2-16d	6-16dx2½"	—	—	1635	1635	1635	—	1465	1465	1465	1, 84, 121, 129, 140
4x6	THA413	18	3½"	13¾"	4½"	4-10d	2-10d	4-10d	—	—	1615	1615	1615	—	1280	1280	1280	1, 84, 121, 140
4x10	THA418	16	3½"	17½"	7½"	4-16d	2-16d	6-16d	—	—	1635	1635	1635	—	1465	1465	1465	1, 84, 121, 140
4x10	THA422	16	3½"	22"	7½"	4-16d	2-16d	6-16d	—	—	1635	1635	1635	—	1465	1465	1465	127
4x10	THAC418	16	3½"	17½"	7½"	4-16d	2-16d	6-16d	—	—	1635	1635	1635	—	1465	1465	1465	127
4x2 truss	THA426	14	3½"	26"	7½"	4-16d	4-16d	6-16d	—	—	2425	2425	2425	—	1940	1940	1940	170
MAXIMUM NAILING — ALL NAIL HOLES FILLED																		
2x4	THA29	18	1½"	9¼"	5½"	—	16-10d	—	4-10d	560	2125	2310	2350	480	1740	1785	1815	1, 84, 140
2x6	THA213	18	1½"	13¾"	5½"	—	14-10d	—	4-10d	930	1795	1840	1870	780	1385	1425	1450	1, 84, 121, 140
2x6	THA218	18	1½"	17¾"	5½"	—	18-10d	—	4-10d	930	1795	1840	1870	780	1385	1425	1450	1, 84, 121, 140
(2)2x10	THA218-2	16	3½"	17¼"	8"	—	16-16d	—	6-16d	1550	2830	3050	3050	1355	2385	2740	2820	1, 84, 121, 129, 140
(2)2x10	THA222-2	16	3½"	22¾"	8"	—	22-16d	—	6-16d	1550	3510	3595	3650	1355	2705	2775	2820	1, 84, 121, 129, 140
4x6	THA413	18	3½"	13¾"	4½"	—	14-10d	—	4-10d	930	1940	2235	2400	780	1660	1910	2075	1, 84, 121, 140
4x10	THA418	16	3½"	17½"	7½"	—	16-16d	—	6-16d	1550	2830	3050	3050	1355	2385	2740	2980	1, 84, 121, 140
4x10	THA422	16	3½"	22"	7½"	—	22-16d	—	6-16d	1550	2830	3050	3050	1355	2385	2740	2980	127
4x10	THAC418	16	3½"	17½"	7½"	—	16-16d	—	6-16d	1550	2830	3050	3050	1355	2385	2740	2980	127
4x2 truss	THA426	14	3½"	26"	7½"	—	30-16d	—	6-16d	1715	4020	4625	4655	1355	3480	4000	4030	170
FACTORY SKEW 45°																		
4x truss	THAR/L422	16	3½"	22¾"	8"	4-10d	2-10d	1-10d	2-10dx1½"	—	1090	1090	1090	—	915	915	915	47, 134
4x truss	THAR/L422	16	3½"	22¾"	8"	4-10d	12-10d	1-10d	2-10dx1½"	310	1675	1675	1675	260	1405	1405	1405	47, 134

1. 16d sinkers may be used to replace 16d commons at 0.85 of table load.
2. Uplift has been increased 33% and 60% for earthquake or wind loading with no further increase allowed; reduce where other loads govern.
3. Roof loads are 125% of floor loads unless limited by other criteria.

4. THAR/L422 with 4-10d top nails and 2-10d face nails: When the hanger height is between 9" to 12", the allowable download is 1440 lbs for DFL and 1210 lbs for SPF. No further increase allowed.
5. NAILS: 16d = 0.162" dia. x 3½" long, 16dx2½" = 0.162" dia. x 2½" long, 10d = 0.148" dia. x 3" long, 10dx1½" = 0.148" dia. x 1½" long. See page 16-17 for other nail sizes and information.

EXHIBIT 17

Solid Sawn Connectors

Catalog C-2006 © Copyright 2005 SIMPSON STRONG-TIE CO., INC.

(C2919FCR-KWOOD / MUD RM / CANT SIDE & BACK -- 240 CANTERBURY - F2)

Top chord 4x2 SP #2
 Bot chord 4x2 SP #2
 Webs 4x2 SP #3

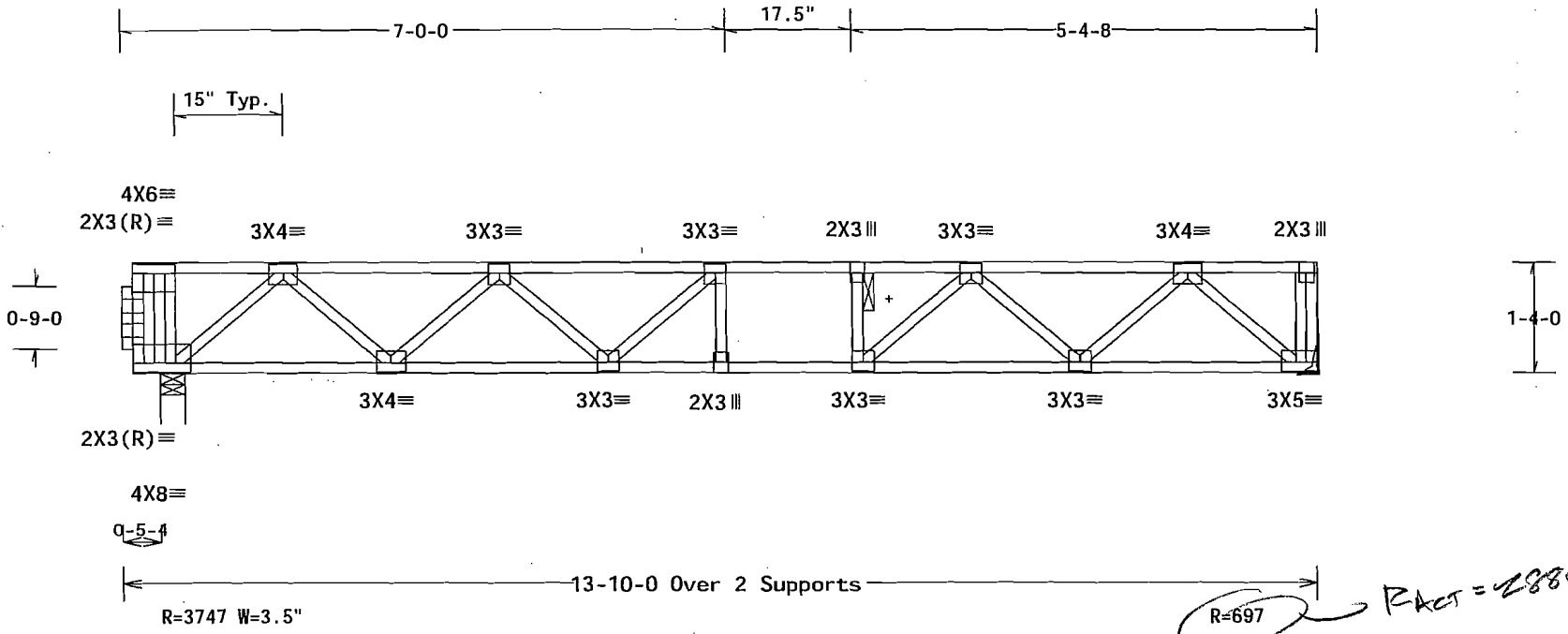
+ 2x6 continuous strongback. See ANSI/TPI 1-02 Sect. 7.5.

This truss spaced @ 24.0" OC supports additional concentrated load at left end from 8.00' stud wall (110.00 PSF) supporting 29-0-0 roof spans with 2-0-0 overhang. Roof load: 20.00 psf LL and 15.00 psf DL.

Deflection meets L/360 live and L/240 total load.

Truss must be installed as shown with top chord up.

EXHIBIT 18

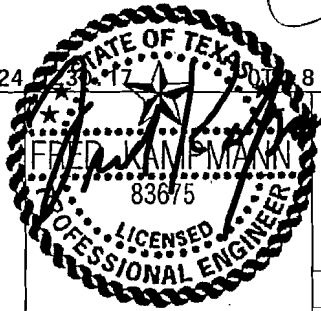


R_{ACT} = 2880 #

PLT TYP. WAVE TPI\R

Design Crit: TPI-1995(STD)

7.24



TX/-/1/-/1-/R/- Scale = .5"/Ft.



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TOT.LL	40.0 PSF	REF R6918- 21815
TC DL	10.0 PSF	DATE 06/18/07
BC DL	5.0 PSF	DRW MOUSR6918 07169066
BC LL	0.0 PSF	MO-ENG FK/FK
TOT.LD.	55.0 PSF	SEQN- 20688
DUR.FAC.	1.00	
SPACING	24.0"	JREF- 1T8B6918Z08

(C2919FCR-KWOOD / MUD RM / CANT SIDE & BACK -- 240 CANTERBURY - F5)

Top chord 4x2 SP #2
 Bot chord 4x2 SP #2
 Webs 4x2 SP #3

+ 2x6 continuous strongback. See ANSI/TPI 1-02 Sect. 7.5.

Deflection meets L/360 live and L/240 total load.

Truss must be installed as shown with top chord up.

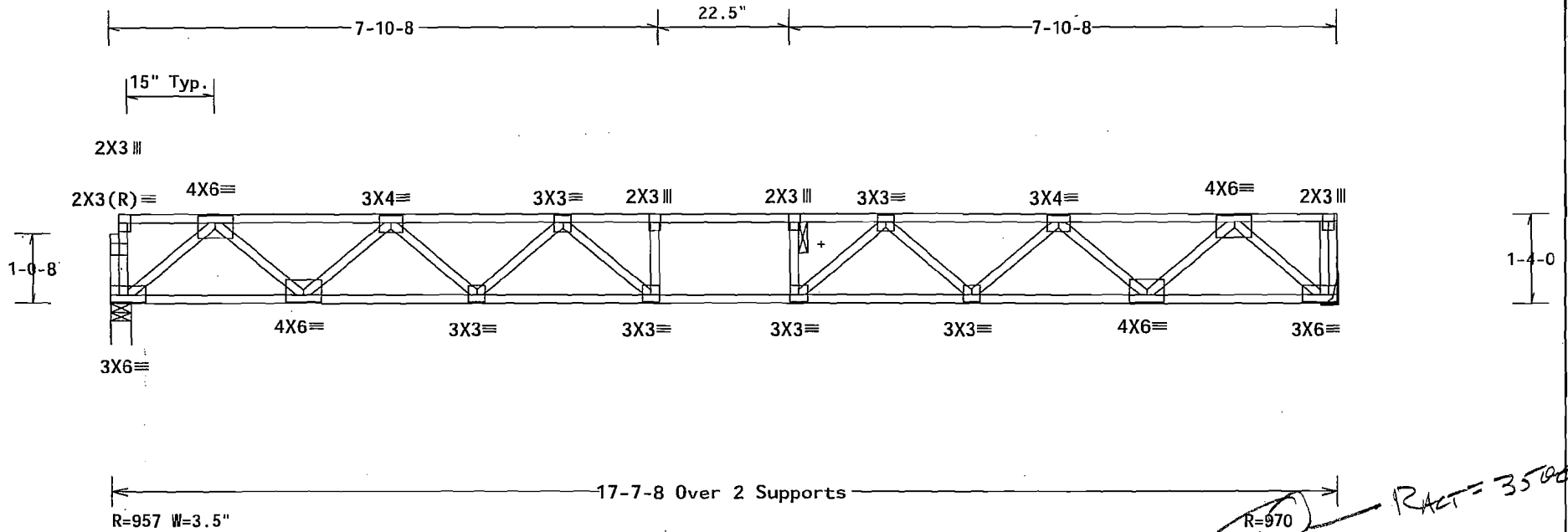
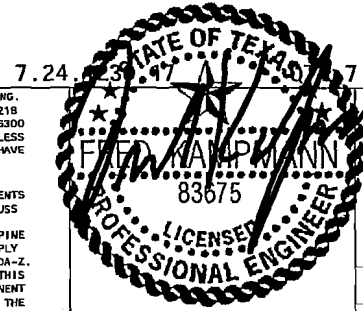


EXHIBIT 19

PLT TYP. WAVE TPI\R

Design Crit: TPI-1995(STD)

7.24



TX/-/1/-/-/R/-

Scale = .45"/Ft.



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TC HL	40.0 PSF	REF R6918- 21818
TC DL	10.0 PSF	DATE 06/18/07
BC DL	5.0 PSF	DRW MOUSR6918 07169064
BC LL	0.0 PSF	MO-ENG FK/FK
TOT.LD.	55.0 PSF	SEQN- 10635
DUR.FAC.	1.00	
SPACING	24.0"	JREF- 1T8B6918Z08

Jun 19 '07

(C2919FCR-KWOOD / MUD RM / CANT SIDE & BACK -- 240 CANTERBURY - F6)

Top chord 4x2 SP #2
 Bot chord 4x2 SP #2
 Webs 4x2 SP #3

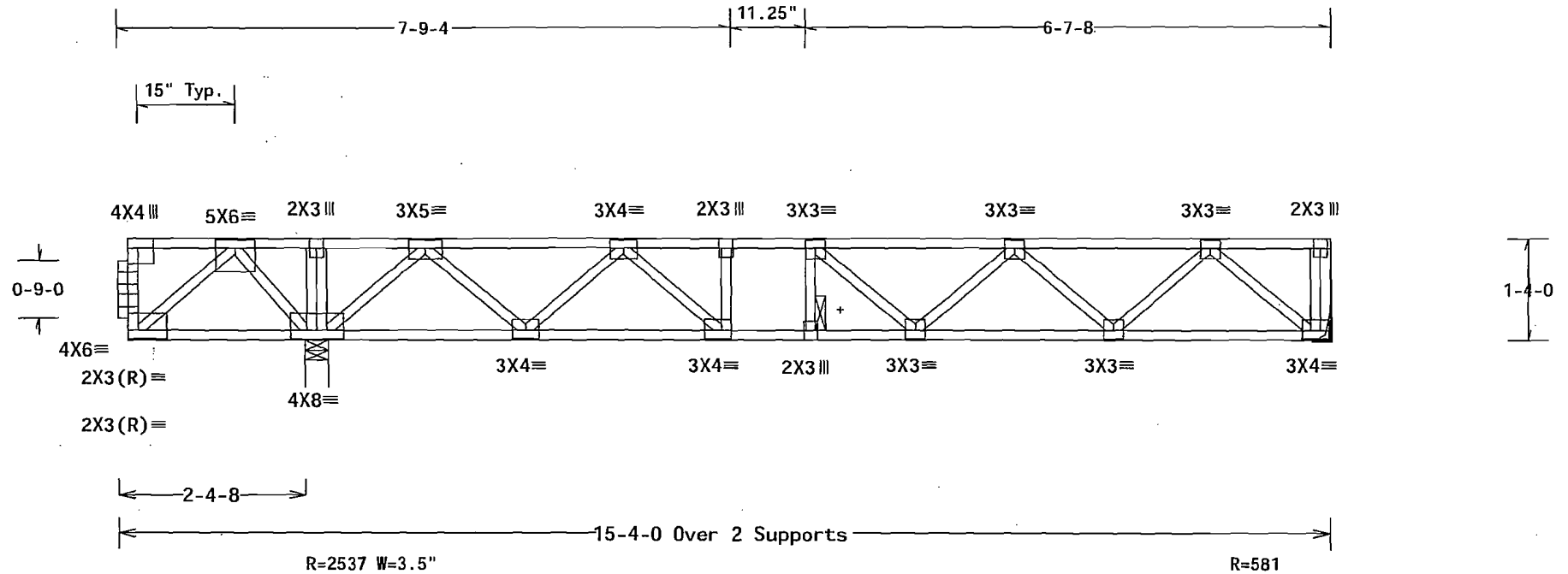
+ 2x6 continuous strongback. See ANSI/TPI 1-02 Sect. 7.5.

This truss spaced @ 24.0" OC supports additional concentrated load at left end from 8.00' stud wall (13.75 PSF) supporting 28-0-0 roof spans with 1-6-0 overhang. Roof load: 20.00 psf LL and 15.00 psf DL.

Deflection meets L/360 live and L/240 total load.

Truss must be installed as shown with top chord up.

EXHIBIT 20



PLT TYP. WAVE TPI\R

Design Crit: TPI-1995(STD)

7.24.250.17

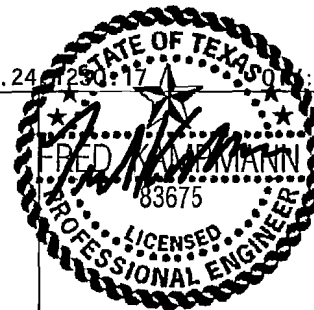
TX/-/1/-/R/-

Scale = .5"/Ft.



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TC LL	40.0 PSF	REF R6918- 21819
TC DL	10.0 PSF	DATE 06/19/07
BC DL	5.0 PSF	DRW MOUSR6918 07170002
BC LL	0.0 PSF	MO-ENG FK/FK
TOT.LD.	55.0 PSF	SEQN- 20709
DUR.FAC.	1.00	
SPACING	24.0"	JREF- 1T8B6918Z08

Top chord 2x8 SP #2
 Bot chord 2x6 SP #2
 Webs 2x4 SP #3

2 COMPLETE TRUSSES REQUIRED

SPECIAL LOADS

----- (LUMBER DUR. FAC. = 1.25 / PLATE DUR. FAC. = 1.25)
 TC - From 56 PLF at 0.00 to 56 PLF at 17.96
 BC - From 14 PLF at 0.00 to 14 PLF at 17.96
 PLB- 1875 LB Conc. Load at (5.08, 20.62)
 PLB- 1102 LB Conc. Load at (7.08, 20.62)
 PLB- 957 LB Conc. Load at (9.08, 20.62)
 PLB- 2471 LB Conc. Load at (11.02, 20.62)

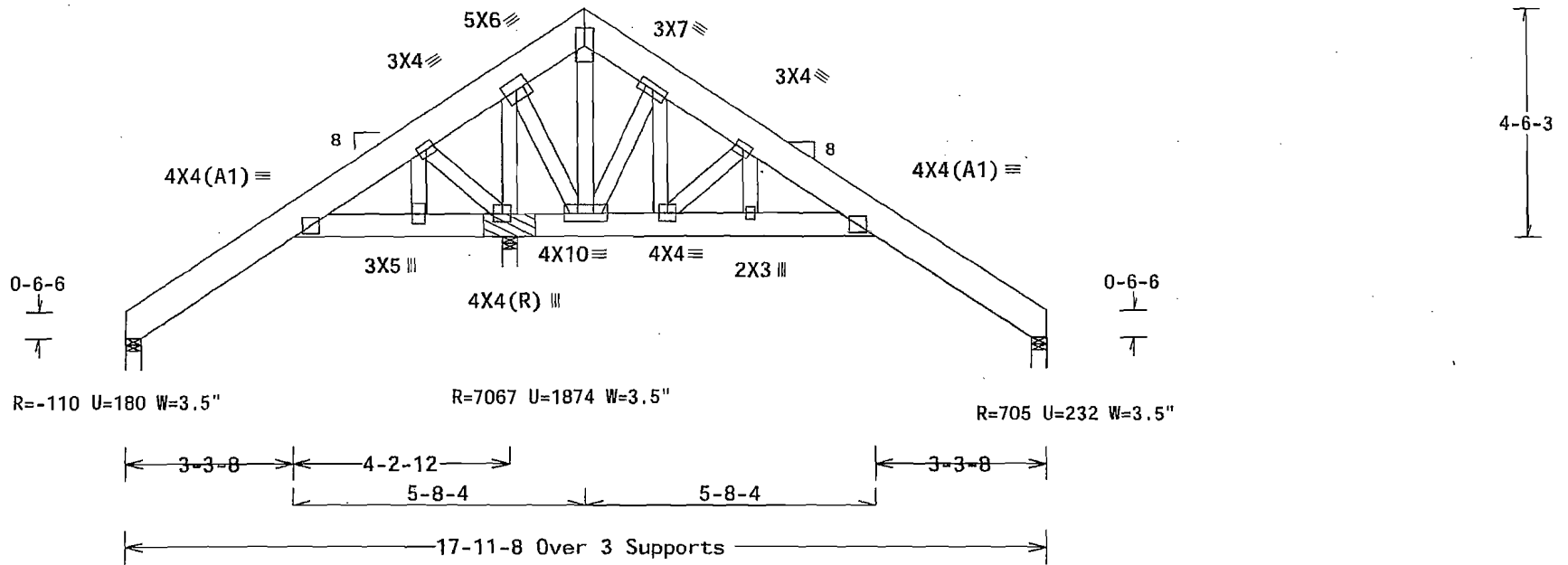
NAILING SCHEDULE: (10d_box_nails)
 TOP CHORD: 1 ROW @ 12" o.c.
 BOT CHORD: 1 ROW @ 12" o.c.
 WEBS : 1 ROW @ 4" o.c.
 USE EQUAL SPACING BETWEEN ROWS AND STAGGER NAILS
 IN EACH ROW TO AVOID SPLITTING.

80 mph wind, 22.11 ft mean hgt, ASCE 7-93, CLOSED bldg, not located
 within 4.50 ft from roof edge, 100 mi from coast, CAT I, EXP C,
 TC DL=5.0 psf, wind BC DL=5.0 psf.

Bearing blocks: Nail type: 10d_box_nails
 BRG X-LOC #BLOCKS LENGTH/BLK #NAILS/BLK WALL PLATE
 2 7.375' 1 12" 5 Match Truss
 Bearing block to be same size and species as bottom chord.
 Refer to drawing CNBRGBLK0503 for additional information.

Deflection meets L/360 live and L/240 total load.

EXHIBIT 23



PLT TYP. WAVE TPI \R

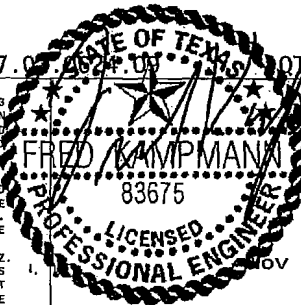
Design Crit: TPI-1995(STD)/UBC

7.08.2004.03.03.01 TX/-/1/-/-/R/-

Scale = .3"/Ft.

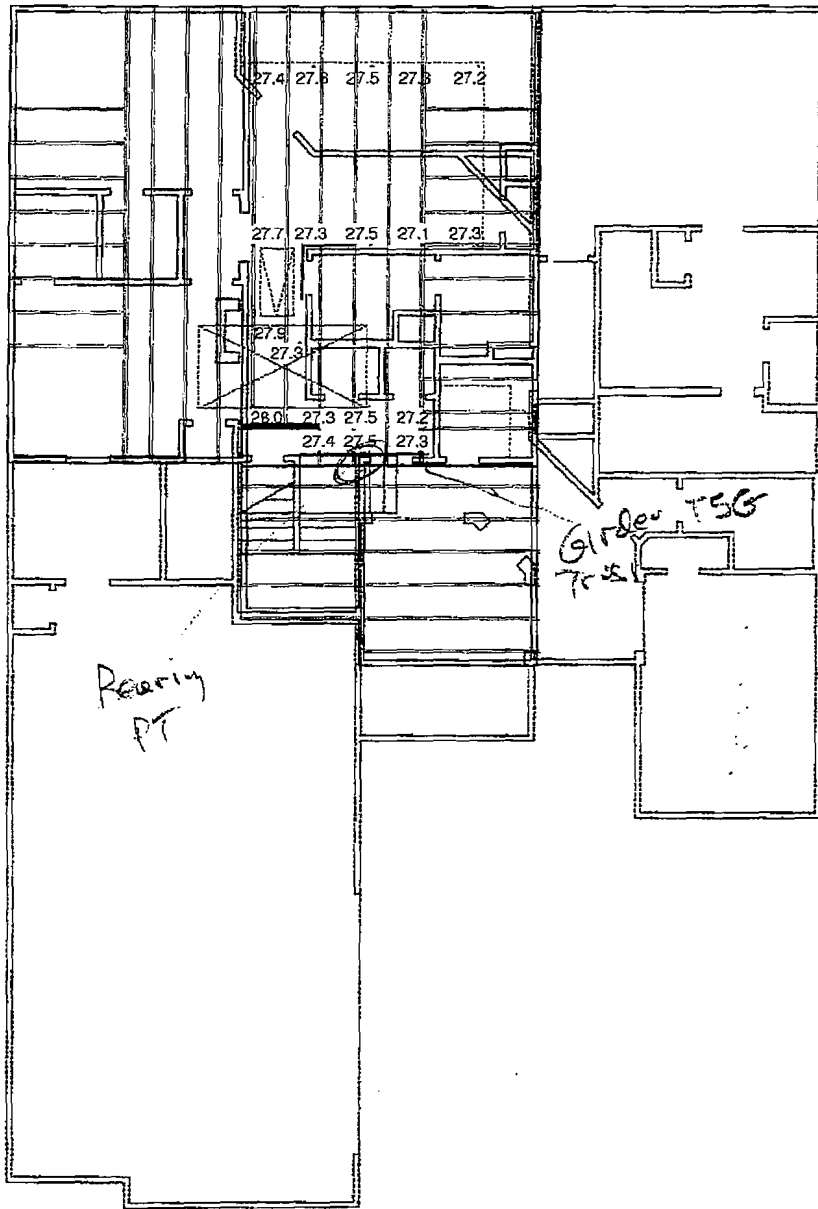
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONDRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 8300 ENTERPRISE LN MADISON, WI 53710) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H/S/K) ASTM A653 GRADE 40/80 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R6918- 65364
TC DL	8.0 PSF	DATE 11/03/04
BC DL	7.0 PSF	DRW MOUSR6918 04308005
BC LL	0.0 PSF	MO-ENG /FK
TOT.LD.	35.0 PSF	SEQN- 23721
DUR.FAC.	1.25	
SPACING	24.0"	





UPPER LEVEL CEILING ELEVATIONS

READINGS IN .25 INCH INCREMENTS

ADDRESS: 240 CANTERBURY DRIVE

JOB No: 714008000.004

CLIENT: CAPITAL PACIFIC

DATE: 4-25-07

SHEET:

GEOSTRUCTURAL/STRUCTURAL
GEOTECHNICAL/PAVEMENTS
FORENSIC

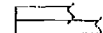


2804 LONGHORN BLVD.
AUSTIN, TEXAS 78768
(512) 835-7000

EXHIBIT 2A

Job: (XX) KWOOD D / CANT REAR / / REV AT A/C / TRAY ENT /

THIS DVG. PREPARED BY THE ALPINE JOB DESIGNER PROGRAM FROM TRUSS MFR'S OUTPUT

Complete Trusses Required 

Top chord 2x8 SP #1
Bot chord 2x6 SP #2
Webs 2x4 SP #3 : V2, W8 2x4 SP #2:

SPECIAL LOADS
--- (LUMBER DUR. FAC. = 1.25 / PLATE DUR. FAC. = 1.25)
TC - From 48 PLF at 0.00 to 48 PLF at 17.96
BC - From 14 PLF at 0.00 to 14 PLF at 17.96
AC - 1718 LB Conc. Load at 5.00
BC - 847 LB Conc. Load at 7.00, 9.00
BC - 1788 LB Conc. Load at 11.12

Deflection meets L/360 live and L/240 total load.

(1) 2x8x7-6-15 SP #1 scab at left end. Attach scab to face of chord with:
10d_Box_or_Gun_(0.128"x3",_min.)_nails @ 8" OC, plus additional nail clusters
at : BRG.: (7), heel: (11), 1st panel point: (4).

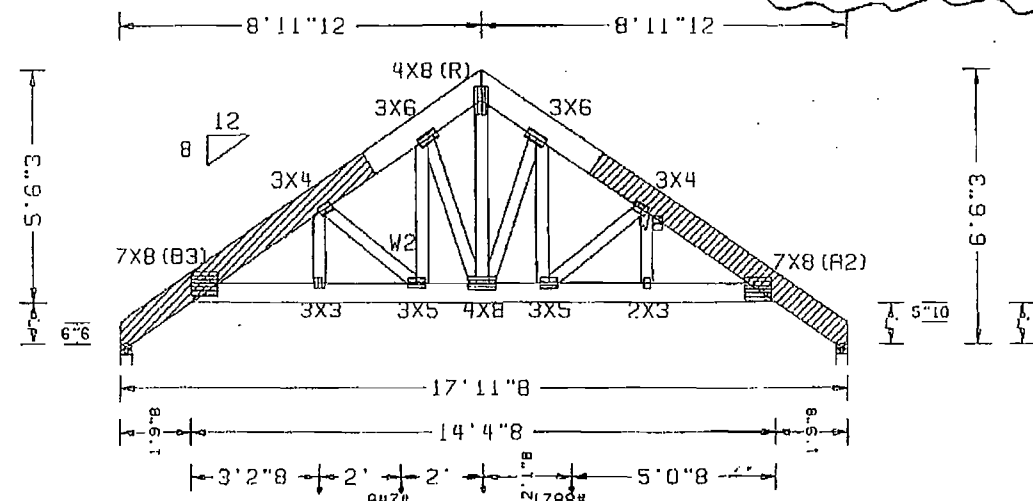
Nailing Schedule: (10d_Box_or_Gun_(0.128"x3",_min.)_nails)
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 5.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.

80 mph wind, 22.11 ft mean hgt, ASCE 7-93, CLOSED bldg, not located within 4.50
ft from roof edge, 100 mi from coast, CAT 1, EXP C, wind TC DL=4.8 psf, wind BC
DL=4.2 psf.

Calculated horizontal deflection is 0.14" due to live load and 0.13" due to dead
load.

(1) 2x8x7-6-15 SP #1 scab at right end. Attach scab to face of chord with:
10d_Box_or_Gun_(0.128"x3",_min.)_nails @ 8" OC, plus additional nail clusters
at : BRG.: (6), heel: (9), 1st panel point: (4).

EXHIBIT 25



*CPH
240 Canterbury
without interior Brg*

Rv=3425# U=1074# W=3"8 Rv=2889# U=906# W=3"8

LEFT RAKE = 1'9"10
LEFT JIG = 9'0"12
TAC = 175
PLT. TYP. WAVE/R

UBC/TPI1995 (STD)

QTY= 1 PLIES= 2 TOTAL= 2

REV. 7.24.1230.17

RIGHT RAKE = 1'9"10
RIGHT JIG = 9'0"12
SEO = 19568
SCALE = 0.2500

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-09 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 O'CONNOR DR., SUITE 200, MADISON, WI 53715) AND WTCR (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.
IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY NDS) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/18GA (V, W/S/K) ASTM A653 GRADE 40/50 (V, W/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 180R-2. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER PAR 13 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER (ANSI/TPI) 1 SEC. 2.

TC LL	16.0psf	REF
TC DL	8.0psf	DATE 05-18-2007
BC DL	7.0psf	DRWG
BC LL	0.0psf	
TOT. LD.	31.0psf	O/A LEN. 171108
DUR. FAC.	1.25	
SPACING	24.0"	TYPE SPEC

May 19 07 09:30a

Buff... Framing Truss Dpt

(S12) 6-6-0966

P. 2

Oct 01 08:05:02P

Ron and Carol Hemphill

512-238-1494

P. 16

Photo Log
240 Canterbury Dr.
Austin, TX 78737-4550
Amstar Engineering Project No. 1954
Photos dated October 10, 2008

Photo No.	Remarks
1	Inspection port IP1. Refer to report text for discussion and Exhibit 15 for location. This photograph shows the 3" x 15-3/4" glue lam beam and two 16" deep floor trusses with diagonal members MPC connected. The floor trusses are supported by Simpson THA 413 steel hangers.
2	Another view of inspection port 1P1.
3	Closer view of the THA 413 steel hangers. Note the vertical leg is not bent over the top of the beam in this photo.
4	View inside the floor cavity. Photograph taken at Inspection port IP2.
5	Inspection port IP2 located new the stair.
6	IP2 location. Photo is taken looking toward the stairway from the living room. The far truss in the photo is the F8G truss. Refer to report text for discussion of this truss. Note the 2x bridging that abuts the truss and is nailed to a vertical block.
7	Another view of the bridging at IP2.
8	Truss F8G showing additional blocks wedged into the truss panel area.
9	View inside the floor cavity. Photograph taken at inspection port IP2.
10	View inside the floor cavity. Photograph taken at inspection port IP2.
11	View inside the floor cavity. Photograph taken at inspection port IP2.
12	View inside the floor cavity. Photograph taken at inspection port IP2.
13	View inside the floor cavity. Photograph taken at inspection port IP2.
14	IP2 location showing blocks wedged in panel area of F8G truss.
15	View inside the floor cavity. Photograph taken at inspection port IP2.
16	View inside the floor cavity. Photograph taken at inspection port IP2.
17	Irregular ceiling surface and ceiling/wall corner intersection repairs.
18	Irregular ceiling surface and ceiling/wall corner intersection repairs.
19	Upstairs Bedroom 4 ceiling.
20	Upstairs Bedroom 4 floor at doorway.
21	Closer view of F8G truss from upstairs Bedroom 4. The truss is marked "1-9-5". Note the beam framing into the truss. This beam does not align with the wall. This entire area appears to be improperly framed.
22	This door jamb at bedroom 4 appears to be exhibiting load-related stress from another truss above.
23	Plywood floor decking measurement at Bedroom 4.
24	Closer view of the beam-truss connection shown in photo 21.
25	Another view of the beam-truss condition at Bedroom 4.